

# Phase-resolved modeling of wave interference and its effects on nearshore circulation in a large ebb shoal-beach system

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## Key Points:

- A time-domain Boussinesq model reproduced wave interference observed at Ocean Beach, CA.
- The model reveals small-scale persistent fingering structures in the wave height distribution tied with nearshore flow structures not predicted in the traditional nearshore circulation model.
- Alongshore-varying wave breakers caused by wave interference are the source of vorticity generation, inducing energetic vortex eddies nearshore.

## Abstract

A time-domain Boussinesq model was applied to modeling wave interference and its effects on nearshore circulation in San Francisco Bar and the adjacent Ocean Beach, CA. The model predicted the wave interference phenomena caused by the ebb shoal, with interference scales consistent with the Radar observation. Model results reveal that small-scale fingering structures in the wave height distribution result from wave interference and are persistent with nodal lines unchanged with time. Nearshore circulation predicted by the model shows small-scale flow structures tied with the wave modulation patterns. However, the small-scale modulation in the wave field seems not to generate alongshore variation in wave setup at similar scales. Therefore, in a large-scale view, the alongshore currents predicted by the Boussinesq model still keep the general features shown in a wave-averaged model, such as the flow divergence caused by the pressure gradient force associated with the alongshore variation of wave setup. The time-domain Boussinesq model predicted the spatial variability of wave-induced processes. The alongshore-varying wave breakers caused by wave interference are the source of the vorticity generation, inducing energetic vortex eddies nearshore.

## Plain Language Summary

Coherent wave interference is a common phenomenon caused by surface wave interaction with seabed topography, nearshore structures, or coastal currents. It cannot be predicted by the conventional wave-averaged model based on the Radiative Transfer Equation. The objective of the study is to use a time-domain Boussinesq model to predict the wave interference at the ebb-tidal delta of San Francisco, CA, which was observed by Radar measurements. The model shows small-scale fingering structures in the wave height distribution resulting from wave interference with alongshore scales consistent with the measurements. The small-scale variation in the wave field is persistent, inducing small-scale flow structures nearshore. However, the wave field modulation seems not to generate alongshore variation in wave setup in the small scales. Therefore, in a large-scale view, the alongshore currents predicted by the Boussinesq model still keep the similar features as shown in a wave-averaged model, such as the flow divergence caused by the pressure gradient force associated with the alongshore variation of wave setup. The time-domain Boussinesq model also shows energetic vortex eddies in the surf zone, which are generated by alongshore varying wave breakers tied with wave interference.

## 1 Introduction

Coherent wave interference commonly occurs in the coastal ocean due to surface wave interaction with the inner-shelf topography, coastal structures, or coastal currents [Smit and Janssen, 2013]. It is more significant for narrow-band waves, i.e., swells, which exhibit persistent wave height variability in the laboratory [Vincent and Briggs, 1989; Chawla et al., 1998] and field [Smit et al., 2016]. Breaking waves under such coherent interference influences can generate nearshore circulation cells, which are more stationary [Dalrymple, 1975; Dalrymple et al., 2011] in contrast to transient circulation cells induced by non-coherent short-crested waves [Johnson, 2004; Spydell and Feddersen, 2009].

Modeling of coherent wave interference and associated wave-induced processes is challenging, especially at field scale. Traditional field-scale wave prediction models, such as SWAN [Booij et al., 1999] or WaveWatch [Tolman, 1991], are stochastic models based on the Radiative Transfer Equation (RTE), which requires the assumptions for a slowly varying medium and statistically independent wave components [Komen et al., 1996]. Such assumptions make the model unable to resolve inhomogeneous wave patterns caused by coherent wave interference. These limitations have been partially alleviated by the recent work of Smit and Janssen [2013], who introduced the second-order statistics into a stochastic wave model, allowing the modeling of statistical wave interference caused by wave refraction and diffraction in coastal seas. The model was validated against in situ data measured during the NCEX

experiment in the Scripps and La Jolla Canyon [Smit *et al.*, 2015], which showed that the model based on the quasi-coherent statistical theory is able to predict statistical interference and associated inhomogeneities in the wave field. More recently, Akrish *et al.* [2020] have extended the quasi-coherent model to include the effect of wave-current interaction.

A more direct way to model wave interference is to use phase-resolving wave models, such as Boussinesq models [e.g., Shi *et al.*, 2012] or non-hydrostatic models [e.g., Ma *et al.*, 2012]. Because wave phase information is explicitly retained, the wave interference caused by wave coherence can be directly modeled. Another advantage in using such a phase-resolving wave model is that the model predicts not only wave interference patterns but also wave-wave interaction and breaking-induced nearshore circulation. However, due to the high computational cost of a phase-resolving model, large-domain field applications of such models have rarely been carried out.

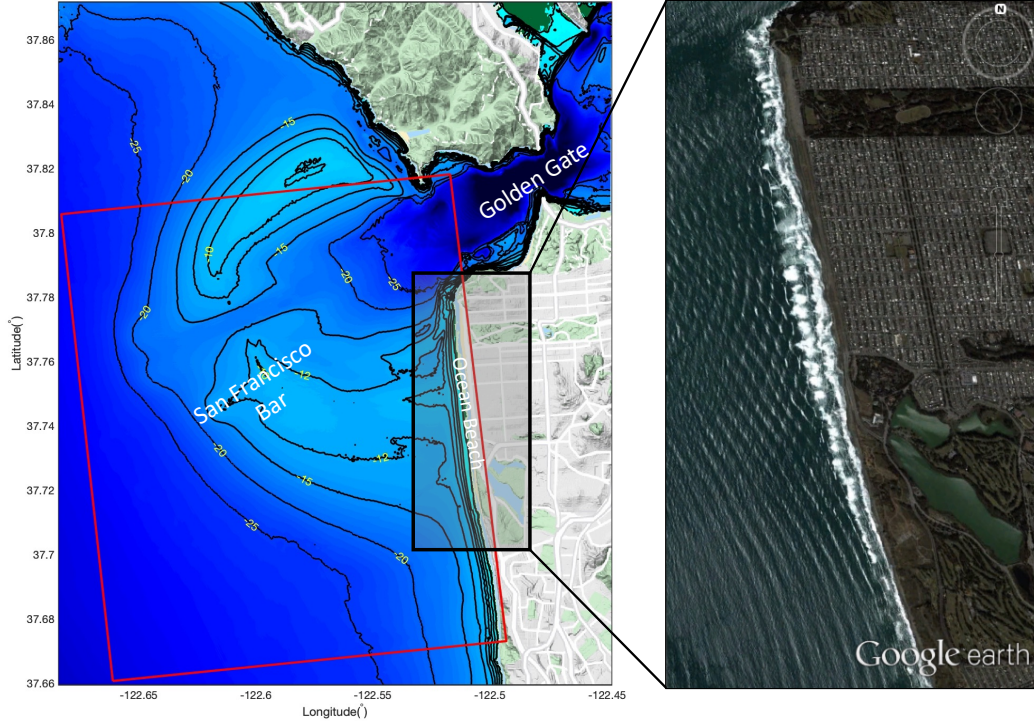
Among the available classes of phase-resolving wave model, Boussinesq-type wave models are less time-consuming than non-hydrostatic wave models using a Pressure Poisson solver [for example, NHWAVE or SWASH Ma *et al.*, 2012; Zijlema *et al.*, 2011]. The parallel computing and the adaptive mesh refinement (AMR) techniques make the model readily feasible to be used for simulations of surface wind waves over domains with dimensions of tens of kilometers [e.g., Chakrabarti *et al.*, 2017].

The objective of this study is to investigate wave interference patterns arising in propagation of nearly unidirectional waves over bathymetry control and small-scale variability in shoreline circulation patterns using a phase resolving model approach. The study site is the San Francisco Bar and the adjacent Ocean Beach as shown in Figure 1.

Figure 1 shows the San Francisco Bar and the adjacent Ocean Beach. Smit *et al.* [2016] reported X-band radar observations which showed strong wave interference patterns in the nearshore region [see Figure 3 in Smit *et al.*, 2016]. Their analysis using the coupled-mode spectrum revealed that, during energetic swell events, the nearshore wave field is comprised of two coherent wave trains originating from the same offshore wave source. Due to wave refraction over the San Francisco Bar, the offshore waves are directionally separated into two swell systems behind the bar, resulting in a coherent interference pattern landward of the bar region. Figure 1 (b) illustrates the wave interference pattern seen in Google Earth imagery taken on March 14th, 2000. The wave height measured at the offshore CDIP buoy 029 (<http://cdip.ucsd.edu>), located at  $37^{\circ}56'45''\text{N}$ ,  $122^{\circ}28'12''\text{W}$ , varied from 1.45 – 2.14 m on that day, with peak wave periods of 9.8 - 16.7 s. The averaged incident angle was  $288^{\circ}$ .

The influence of the San Francisco Bar on nearshore hydrodynamics and sediment transport has been investigated by many researchers. The San Francisco Bar is a large-scale horseshoe-shaped ebb-tidal delta as shown in Figure 1 (a). Earlier studies have shown the effect of wave focusing by the southern shoal, resulting in alongshore variations of wave energy and wave set-up along the adjacent Ocean Beach [Eshleman *et al.*, 2007; Shi *et al.*, 2011; Barnard *et al.*, 2012; Hansen *et al.*, 2014]. The alongshore pressure gradient caused by wave set-up variation is the dominant force in the alongshore momentum balance, driving nearshore circulation and sediment transport on the beach [Shi *et al.*, 2011; Hansen *et al.*, 2013, 2014].

Numerical models used in previous studies of this region have been based on coupled wave and circulation models, such as the Nearshore Community Model [NearCoM, Shi *et al.*, 2005; Chen *et al.*, 2014] and Delft3D [Lesser *et al.*, 2004]. The wave component in both models mentioned above is the wave model SWAN [Booij *et al.*, 1999]. Wave interference and its effect on nearshore circulation could not be modeled due to the phase-averaging nature of the model theory. Our questions are 1) how does the wave interference affect the wavefield in the nearshore region of Ocean Beach; 2) how does the wave interference affect wave-induced circulation; 3) what is the difference in model prediction between a phase-averaging model and phase-resolving model.



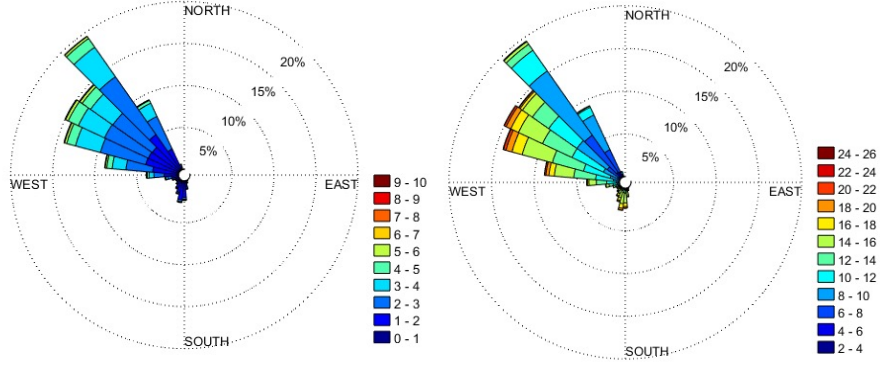
**Figure 1.** (Left) the ebb tidal delta (San Francisco Bar) and Ocean Beach. The computational domain is denoted by the red block. Solid black lines are depth contours. (Right) interference patterns of swells at Ocean Beach observed in the Google Earth imagery taken on March 14th, 2000 (map ©2021 Google, ©TerraMetrics).

To address these questions, we carried out a numerical study using the time-domain Boussinesq wave model, FUNWAVE-TVD, in a large-scale domain (10's km) to cover the entire wave propagation and evolution process, including wave shoaling, refraction, interference, breaking, and wave-induced nearshore circulation. The numerical results were compared with results from the coupled wave and circulation model, NearCoM, which cannot predict wave interference and its nearshore effects.

In the rest of the paper, section 2 briefly reviews the study site and hydrodynamic conditions, emphasizing the wave interference phenomenon caused by the southern shoal. In section 3, the numerical model and model setup are described. The basic characteristics of wave interference are presented using a monochromatic wave simulation and wave ray tracing analysis in section 4. Section 5 provides results of the wave field predicted by the Boussinesq model with comparisons with the measured data and model results from the phase-averaged model, SWAN. In section 6, wave-averaged processes are discussed according to the comparisons between the Boussinesq model and the phase-averaged circulation model. Findings are summarized in section 7.

## 2 Wave interference caused by San Francisco Bar

San Francisco Bar is a large-scale ebb-tidal shoal offshore of the Golden Gate of the San Francisco Bay as shown in Figure 1. The minimum depths are about 9.5 m and 10.5 m on the northern and southern lobes, respectively. Shoreward of the south lobe is Ocean Beach, a 7-km long sandy beach located along the western, ocean-facing boundary of the city of San Francisco. The wave climate of Ocean Beach is characterized by large-scale wave



**Figure 2.** Wave statistics using the measured data from 1996-2021 at CDIP buoy 029 buoy. (Left) wave height (m) and dominant wave angle. (Right) wave peak period (s) and dominant wave angle.

height variability along the beach due to wave refraction over the shoal (Shi et al., 2011, Hansen et al., 2014). Nearshore circulation at this larger scale is governed by the alongshore pressure gradient generated by the variation in wave setup. It is also affected by the tide-induced alongshore pressure gradient due to the presence of the Golden Gate inlet (Hansen et al., 2013). Because Ocean Beach is directly exposed to energetic ocean swells, strong wave interference patterns occur at Ocean Beach, as reported by Smit et al. (2016). The wave interference causes persistent wave height variability at the Ocean Beach, the effect of which on nearshore circulation is unknown.

Ocean swell with periods ranging from 11.4s - 26.6s [O'Reilly et al., 2016] in this region is typically generated by large winter Pacific Ocean storms, which arrive from north-west to west. Lower-energy south swell events can occur during the summer, with a lower probability than the winter swell. Figure 2 shows plots of wave height (left) and dominant wave angle (right) analyzed using data measured during 1996 – 2021 at CDIP buoy 029 (<http://cdip.ucsd.edu>), located at  $37^{\circ}56'45''\text{N}$ ,  $23^{\circ}28'12''\text{W}$ . According to the data, more than 42% of wave events are swell waves, and about 70% of swell come from NWW with the averaging direction of  $295^{\circ}$  and an average period of 14.4 s.

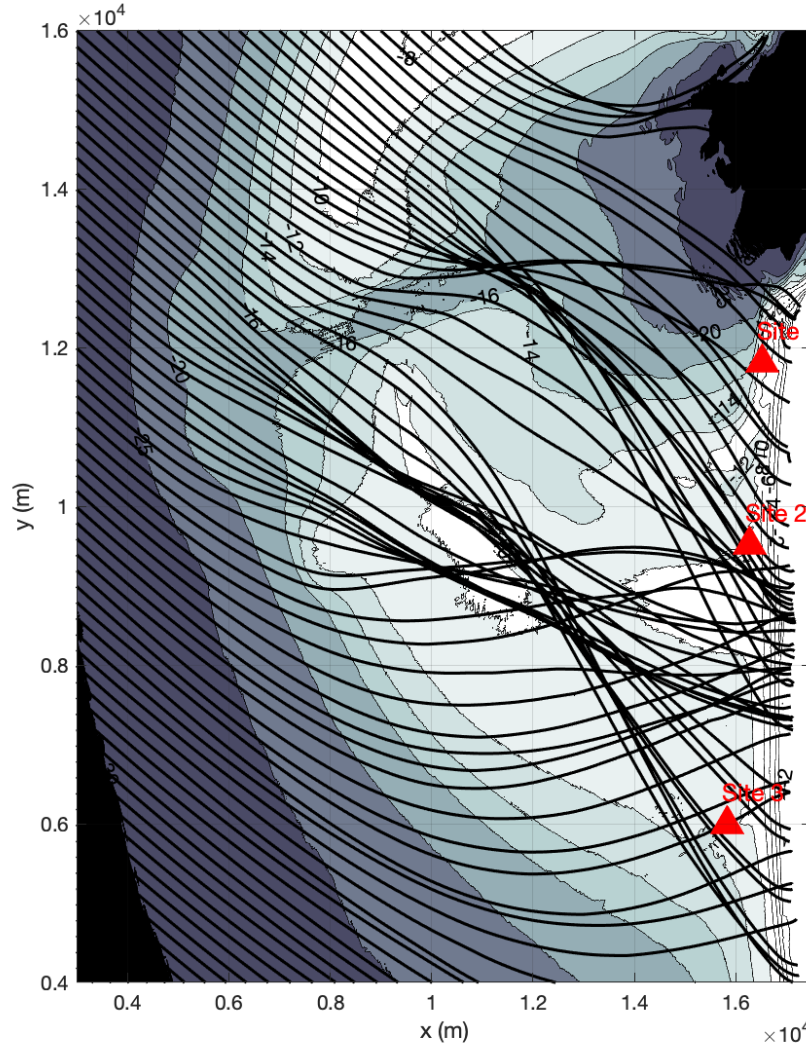
Figure 3 shows the wave ray tracing for offshore swell with a direction of  $300^{\circ}$  and a period of 15 s propagating over the San Francisco Bar and onto Ocean Beach. To facilitate the model setup described in the next section, we rotated the coordinates by  $7^{\circ}$  counter-clockwise to make the shoreline align with the y-direction in the local Cartesian coordinates ( $x, y$ ). As shown in the figure, wave refraction over the shoal causes strong wave interference with significant wave-ray crossing at the nearshore region.

During the summer of 2005 and the winter of 2006, two field experiments were performed to examine spatial differences in wave and current patterns offshore of Ocean Beach (Barnard et al., 2007, Shi et al., 2011). Site 1 – Site 3 are three measurement sites aligned approximately alongshore in water depths from 7 to 13 meters. Detailed deployment locations and durations are referred to Shi et al. (2011).

### 3 Time-domain Boussinesq wave model

FUNWAVE is a widely-accepted public domain Boussinesq-type wave model in the nearshore and tsunami research community. The original generation of the code was developed by Kennedy et al. [2000] and Chen et al. [2000], based on the fully nonlinear Boussinesq equations of Wei et al. [1995]. Subsequent improvements in the formulation covering





**Figure 3.** Wave ray tracing for offshore waves with an incident angle of  $\theta = 300^\circ$  and a period of  $T = 15s$ . Measurement locations, Site 1 - Site 3 are denoted by red triangles. The wave ray crossing appears south of the nearshore measurement location, Site 2.

improvements in nonlinear mode-coupling [Kennedy *et al.*, 2001] and treatment of the vertical component of vorticity [Chen *et al.*, 2003; Chen, 2006] have been incorporated in the recent development of a Total Variation Diminishing (TVD) version, FUNWAVE-TVD, motivated by recent needs for modeling surfzone dynamics, tsunami waves, and coastal inundation processes in both global and coastal scales [Shi *et al.*, 2012]. The model has been parallelized using the Message Passing Interface (MPI) and optimised for use on large-scale, high-performance computing system [Lam *et al.*, 2018]. Further extensions incorporating GPU-acceleration techniques [Yuan *et al.*, 2020] will further increase the feasibility of the model's use for large-domain and long-time simulations. For irregular wave applications, the model is equipped with an internal wavemaker that can generate monochromatic or directional spectral waves based on bulk parameters or measured spectral data. Recently, the

wavemaker has been revised to address the problem of spurious spatial correlations occurring in irregular waves, resulting from discretization of target spectra over a uniform grid of frequencies and alongshore wavenumbers [Salatin *et al.*, 2021].

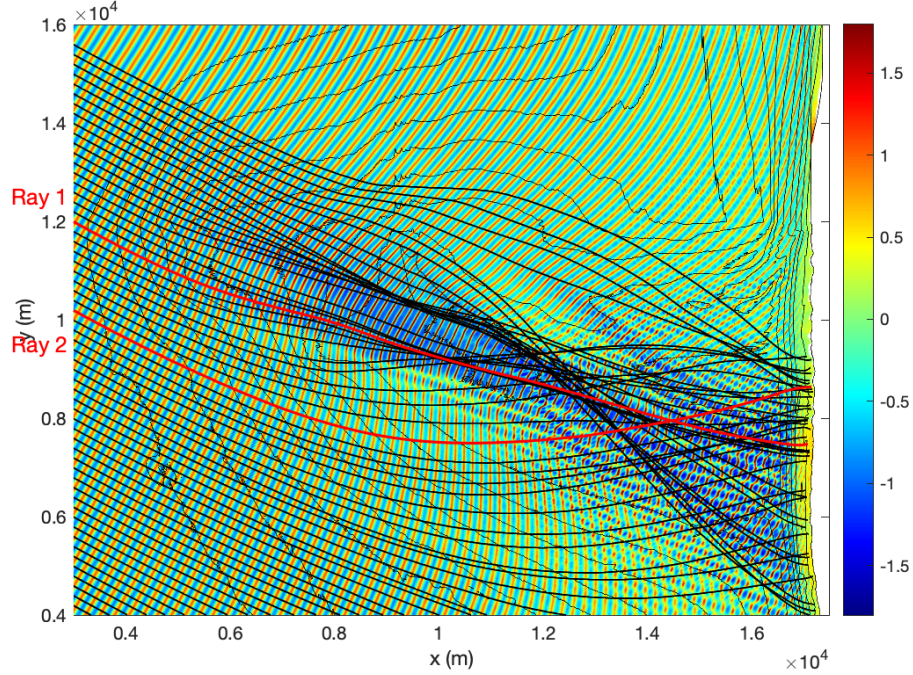
According to the model applicability related to the weakly dispersion assumption,  $kh \leq 3$  [Kirby, 2016], we confine the computational domain nearshore up to the water depth of 35 m, covering the southern ebb-tidal shoal and Ocean Beach, as shown in Figure 1 (red block). The computational grid was constructed using the 10-m digital elevation model (DEM) [Fregoso *et al.*, 2017], which combines bathymetry and topography data with the vertical datum of NAVD88. The mean tide level (MTL) was converted based on the datum at NOAA station 9414290 in San Francisco, CA. The grid has  $8712 \times 7960$  grid points with grid sizes of  $2 \times 2$  m, covering a region of 17.424 km and 15.920 km in the  $x$ - and  $y$ -directions, respectively.

We applied the internal wavemaker, which can generate monochromatic and spectral waves for given directional spectra (e.g., TMA, JONSWAP spectra, or measured spectral data). The wavemaker is located at a 35-m flat bottom (truncated near 35 m contour), 800 m away from the west boundary. A 500m-wide sponge layer was specified at the western boundary to absorb waves propagating seaward from the internal generation region. To apply the lateral periodic boundary condition, we modified the bathymetry and topography at the inlet region to make the grid cyclic in the south-north direction with a 4-km wide buffer region on the north side. Such a reconstruction of the grid does not affect modeled wave field nearshore of Ocean Beach because wave directions considered in this study are basically from NWW. A constant bottom drag coefficient of  $C_d = 0.002$  in the quadratic friction formula was applied in all cases below.

#### 4 Basic characteristics of wave interference in a monochromatic wave field

To demonstrate the basic characteristics of coherent wave interference caused by the tidal ebb shoal, we carried out a simulation of monochromatic waves with an incident angle of  $\theta = 300^\circ$ , a period of  $T = 15s$ , and wave height of  $H = 2m$ , which are typical bulk parameters for winter swells. Figure 4 provides a snapshot of wave surface elevation with wave ray tracing (thick lines) superimposed on the wave surface. Due to the modification for the periodic boundary condition mentioned above, the bathymetric contours in the figure are slightly different from that shown Figure 3 in the northern region. For this reason, the wave rays in the north of the domain are not shown. The figure shows that, before reaching the shoal, waves are regular plane waves with approximately uniform wave height and direction. Wave shoaling effects are significant as waves climb over the shoal, as indicated by large peaks (sharp red color) and lower troughs (blue color) on the shoal. Wave refraction on the shoal causes convergence and divergence of wave rays, inducing larger wave heights in the focusing zone and lower wave heights in the shadow zones (both southern and northern sides of the focusing zone). Wave diffraction patterns are also shown in the shadow zone. Further nearshore, standing wave patterns caused by the wave interference appear in the wave-ray crossing region, which covers a similar area as the wave focusing zone. Waves break near the shoreline, inducing large wave setup as indicated by the warmer color in the surf zone, consistent with the wave setup patterns modeled by Shi *et al.* [2011].

Figure 5 shows the same snapshot as Figure 4, but are 1-D plots along two wave rays denoted by the red lines, Ray 1 and Ray 2, respectively, in Fig. 4. Ray 1 traces waves across the top of the shoal, while Ray 2 is located south of Ray 1, passing the shadow zone. Along Ray 1, wave shoaling occurs as waves enter the shallow shoal (see bathymetric profile (b),  $x < 10,000m$ ). As waves continue propagating shoreward, a large variation in wave amplitude appears though the water depth shoreward is relatively flat (see b). The large variation in amplitude results from the standing wave patterns as revealed in Figure 4. In contrast to Ray 1, waves propagating along Ray 2 do not show shoaling-induced wave height increase because waves along Ray 2 are inside the shadow zone and the seabed slope is milder com-



**Figure 4.** A snapshot of wave surface elevation (color) modeled for offshore monochromatic waves with an incident angle of  $\theta = 300^\circ$ , a period of  $T = 15s$ . Thin solid lines are bathymetric contours, thick solid lines denote wave rays. The red solid lines represent two wave rays selected for 1D surface elevation plots in Fig. 5.

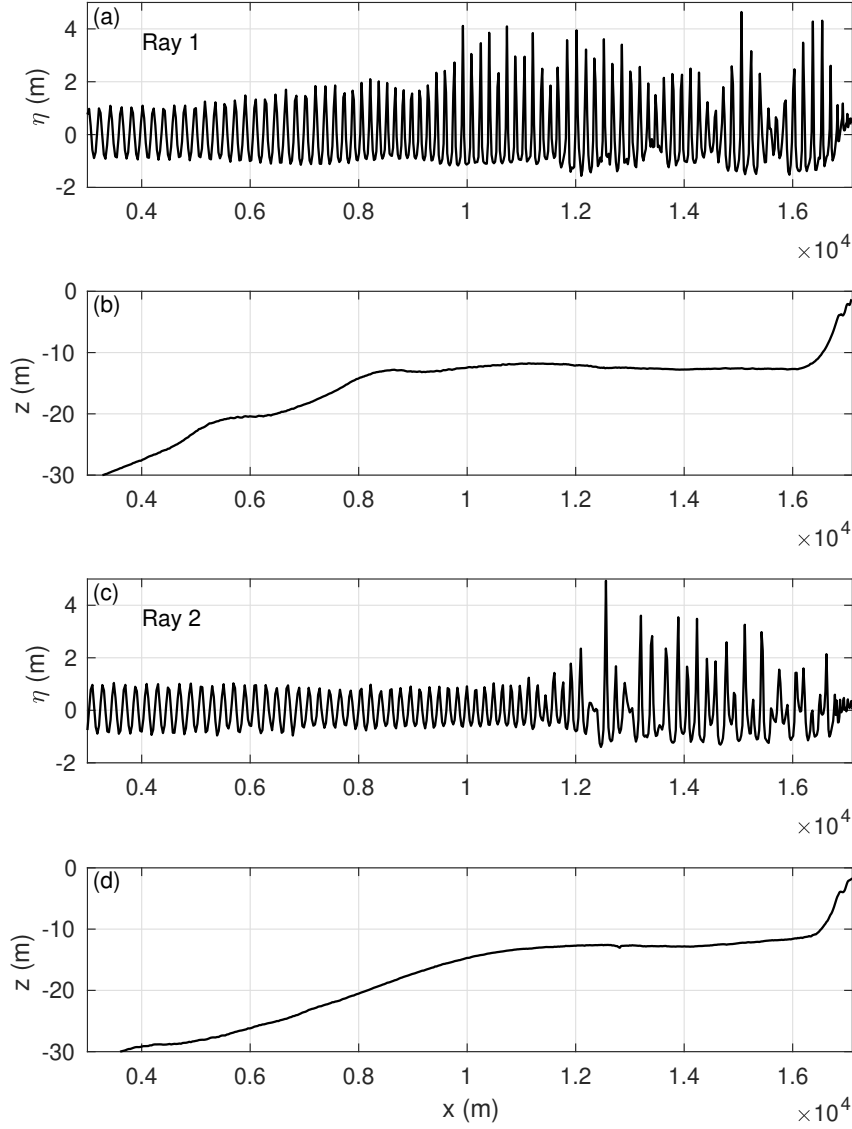
pared to the profile along Ray 1. However, large waves emerge nearshore ( $x > 12,000m$ ) as the wave ray bends northwards, entering the wave interference region.

The alongshore modulation scales can be represented by sizes of standing wave cells at a certain water depth. They can be calculated by the wave length and the wave cross-  
ing angles obtained from wave ray directions. Figure 6 shows alongshore sizes of standing wave cells at 10 m water depth offshore of Ocean Beach calculated based on wave tracing for waves of  $T_p = 15$  s with different incident angles. It appears that the sizes of standing wave cells do not vary much with the incident wave angles, except for the incident angle of  $165^\circ$ , in which wave interference occurs at the northern part of Ocean Beach. The sizes are in 150 ~ 310 m, which is generally consistent with the alongshore sizes of 100 ~ 300 m observed in Smit *et al.* [2016]. The size may change with the incident wave period and water depth.

## 5 Spectral wave field

Based on the availability of field data, we selected two wave conditions that occurred during the observation periods in summer 2005 and winter 2006, respectively, both of which favor the generation of wave interference patterns in terms of wave incident angle. Case 1 simulates the wave condition appeared on July 12, 2005 with the significant wave height  $H_{sig} = 2.49$  m, the peak period,  $T_p = 11.1$  s, and dominant wave angle  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy. Case 2 is a major storm event occurred on January 4, 2006 with  $H_{sig} = 5.1$  m,  $T_p = 14.29$  s, and  $\theta_p = 297^\circ$ . The wave input conditions for the Boussinesq model are based on wave bulk parameters resulting from the large-domain model, NearCoM [Shi *et al.*, 2011]. The JONSWAP spectrum with  $\gamma = 3.3$  and the directional spreading parameter  $\sigma_\theta = 10^\circ$  was partitioned into 1125 wave components, which are randomly phased with





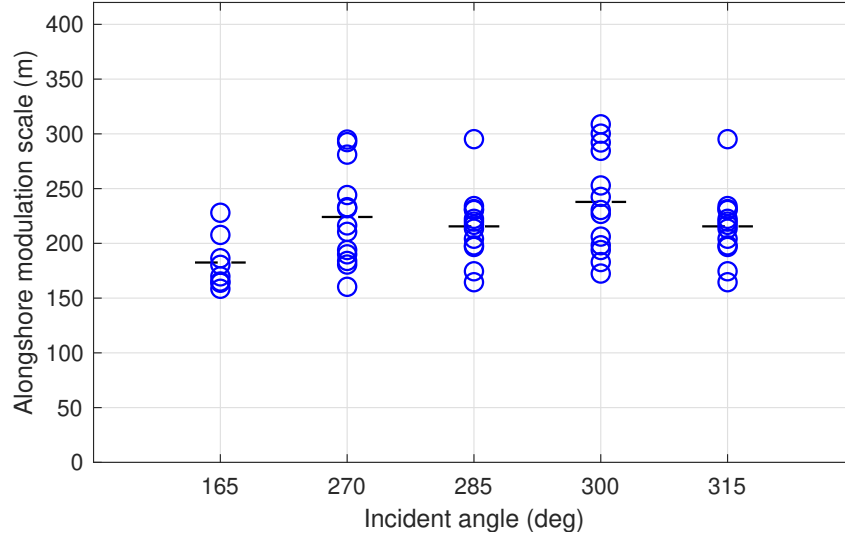
**Figure 5.** Snapshots of wave surface elevation (a, c) and bathymetric profiles (b, d) along the two wave rays selected in Figure 4.

zero-coherence [Salatin *et al.*, 2021]. The water depth is adjusted based on the tidal level recorded at the NOAA station 9414290.

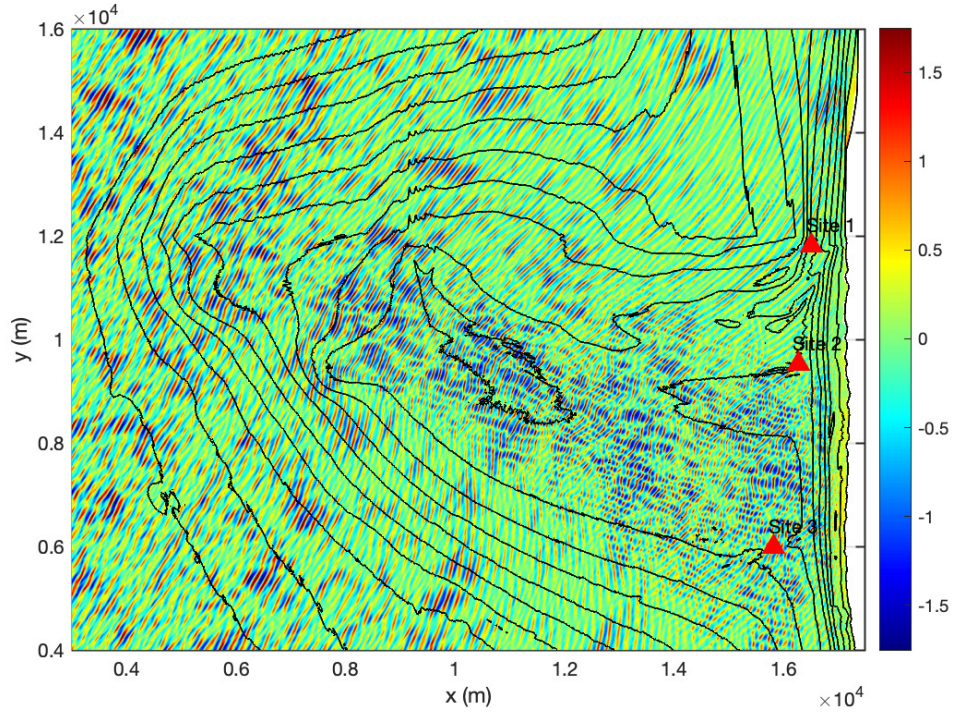
### 5.1 FUNWAVE results

Figure 7 shows a snapshot of surface elevation after a spin-up period of 1800 s. The figure shows wave shoaling at the ebb-shoal, focusing due to wave refraction, and remarkable standing wave features caused by wave interference, generally consistent with the regular wave case. The wave interference concentrates at the nearshore region south of Site 2, similar to the regular wave case due to the similar offshore incident angle.

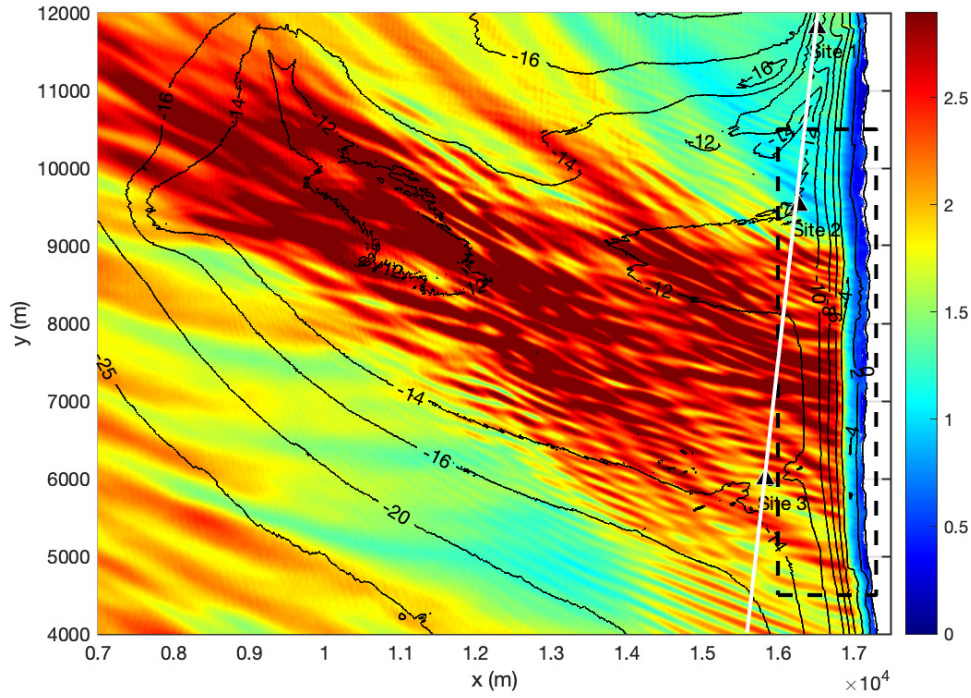
Significant wave height was estimated using the zero-crossing method for the time period of 1800 – 3000 s in the simulation. As shown in Figure 8, large wave heights occur at the ebb-shoal region and the nearshore area behind the shoal due to wave focusing. In partic-



**Figure 6.** Alongshore sizes of standing wave cells at 10 m water depth calculated from wave tracing for offshore waves with different incident angles (Nautical Convention) and  $T_p = 15$  s. Short black lines represent the values averaged over interference rays passing 10 m water depth contour.



**Figure 7.** A snapshot of wave surface elevation (color) for Case 1 simulated by Boussinesq model. The wave condition occurred on July 12, 2005 with  $H_{sig} = 2.49$  m,  $T_p = 11.1$  s, and  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy. Red triangles represent measurement locations, Site 1 - 3 [Shi *et al.*, 2011; Hansen *et al.*, 2013].

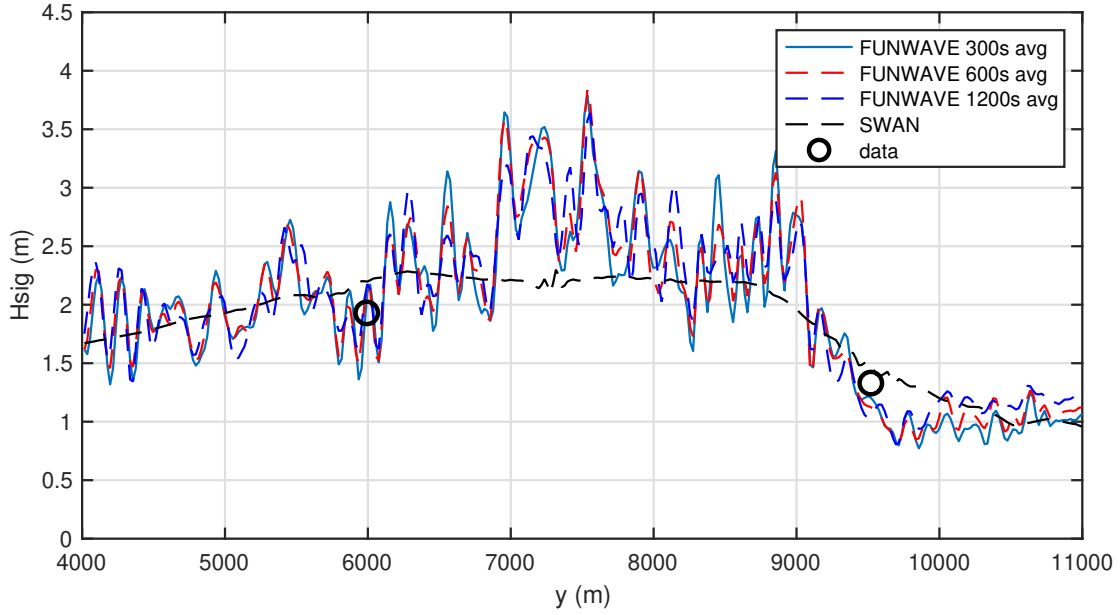


**Figure 8.** Distribution of the significant wave height estimated using zero-crossing in a time interval of 1200 s (1800 s - 3000 s). Case 1:  $H_{sig} = 2.49$  m,  $T_p = 11.1$  s, and  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy. Black triangles represent measurement locations, Sites 1 - 3. The rectangle bounded by black dashed lines is the region shown in Figure 12. The white line is the transect crossing the measurement locations, Site 1 – Site 3.

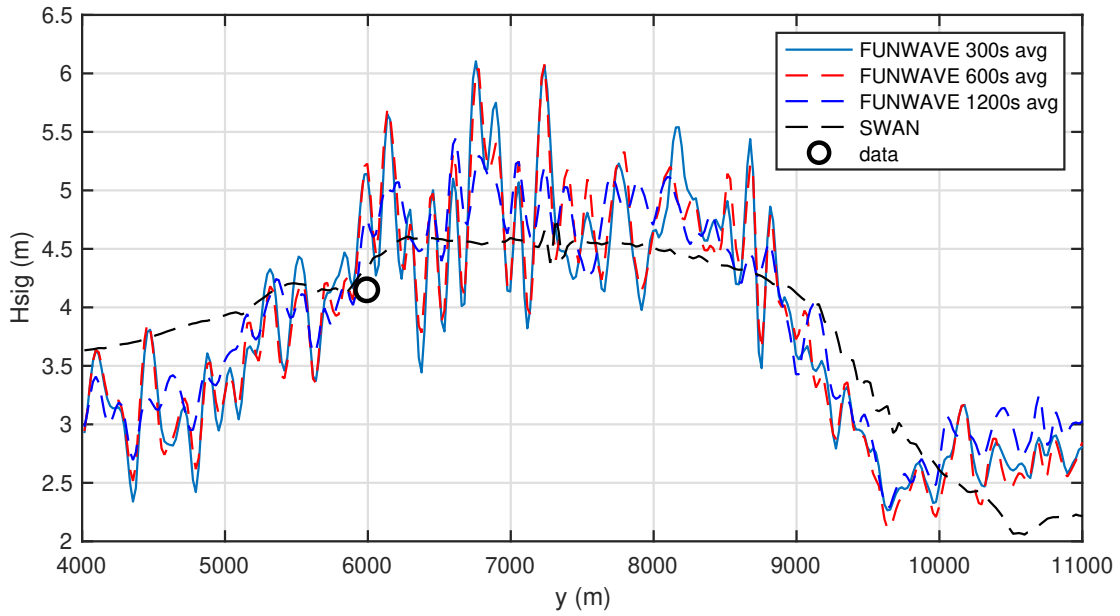
ular, wave height in the nearshore region exhibits fingering patterns arising from the oblique intersection of component waves, as in the monochromatic case, with magnitudes fluctuating in the alongshore direction.

Figure 9 compares the modeled wave height with data along the wave measurement transect denoted by the white line in Figure 9. To examine the persistence of the wave height distribution, we calculated the wave height in the different averaging periods, 300 s, 600 s, and 1200 s, respectively, in the comparison. It is shown that, despite the different averaging periods used for wave height estimate, the recurring modulation patterns are similar with nodal locations approximately fixed, suggesting a pronounced persistent feature associated with wave interference. The magnitudes of fluctuation (from node to antinode) are around 1 m, with the maximum magnitude appearing at the center of the focusing region. Because the data at Site 1 is unavailable, the modeled wave heights are compared with the data at Site 2 and Site 3, respectively, with good agreement. The comparison also shows that the measurements are too sparse in space to capture the wave height fluctuation patterns.

Case 2 is a major storm condition with  $H_{sig} = 5.1$  m,  $T_p = 14.29$  s, and  $\theta_p = 297^\circ$  observed at CDIP 029 buoy. Because the wave incident angle is close to Case 1, the wave propagation and evolution patterns are similar to Case 1 and are thus not shown here. Figure 10 shows the same plot as Fig. 9 but for Case 2 (Note: the data is unavailable at Sites 1 and 2). Again, the fluctuation patterns in the alongshore wave height distribution are quite persistent, reflected by the fixed nodal locations obtained from different averaging periods.



**Figure 9.** Alongshore wave height distributions estimated for averaging periods with durations of 300 s, 600 s, and 1200 along the wave measurement transect, Site 1 - Site 3. Case 1:  $H_{sig} = 2.49$  m,  $T_p = 11.1$  s, and  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy. The dashed line represents the result from the SWAN model, discussed in section 5.2.



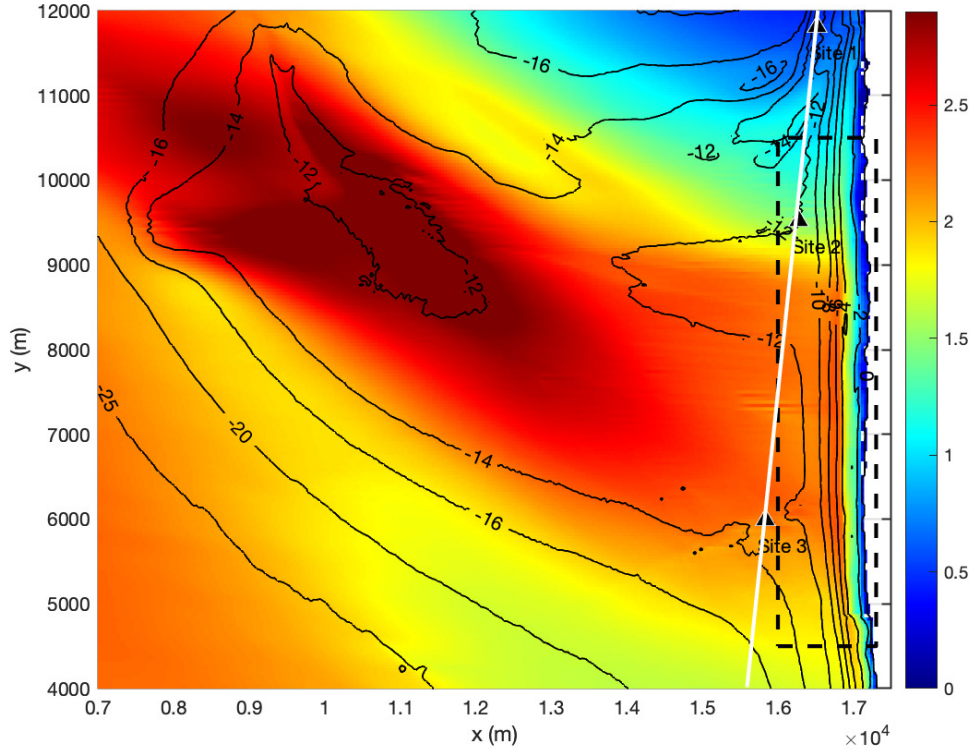
**Figure 10.** The same plot as Fig. 9, but for Case 2.  $H_{sig} = 5.1$  m,  $T_p = 14.29$  s, and  $\theta_p = 297^\circ$  measured at the CDIP 029 buoy.

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The magnitudes of the wave height modulation reach to 2 m, much larger than that in Case 1, expecting a stronger effect on nearshore circulation.

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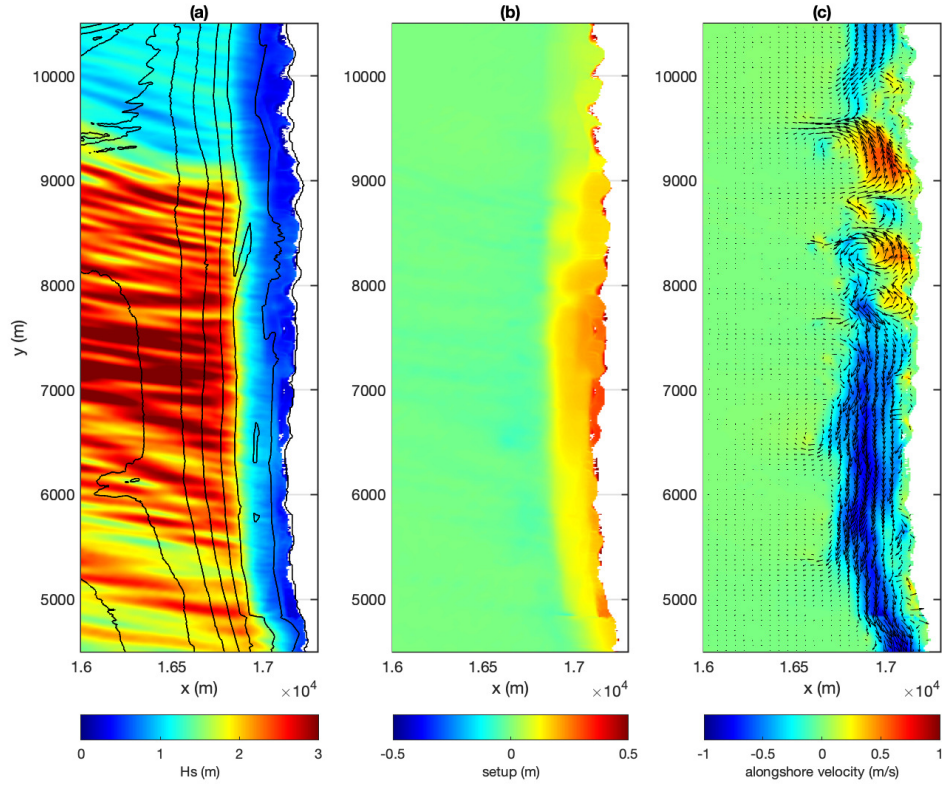


**Figure 11.** Wave height distribution from the SWAN model. Case 1:  $H_{sig} = 2.49$  m,  $T_p = 11.1$  s, and  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy. The rectangle bounded by black dashed lines rectangle is the region shown in Figure 14. The white line is the transect crossing the measurement locations, Site 1 – Site 3.

## 5.2 Comparison to the SWAN model

It is interesting to compare the phase-resolving Boussinesq model and the conventional phase-averaged wave model that cannot model wave interference. The wave model in the previous studies of San Francisco Bar is the SWAN model, such as in NearCoM [Shi *et al.*, 2011] and Delft-3D [Hansen *et al.*, 2013]. Here, we applied the NearCoM model with the same bathymetric data (modified periodic bathymetry) and the same wave conditions as FUNWAVE. To make the model comparable to FUNWAVE, we only considered the surface wave boundary condition, ignoring tidal current, wind, and other external forcing used in Shi *et al.* [2011]. A uniform rectangular grid was used with a grid resolution of 20 m.

Figure 11 shows the wave height distribution from the SWAN model. Wave focusing causes large wave heights mainly distributed on the shoal. Compared to the FUNWAVE results shown in Figure 8, the major difference between the two models can be found in the nearshore region, where FUNWAVE predicts the fingering patterns of the wave field, while SWAN provides a smooth distribution of wave height. A striking difference can be seen in the 1D plot of wave height comparisons along the measurement transect shown in Figures 9 and 10. The SWAN model does not predict the large magnitude fluctuations as in the FUNWAVE model. In addition, FUNWAVE predicts generally larger wave heights at the center of the focusing region than SWAN, probably due to different wave dissipation mechanisms in FUNWAVE, similar to the finding in Geiman *et al.* [2011] who also compared FUNWAVE

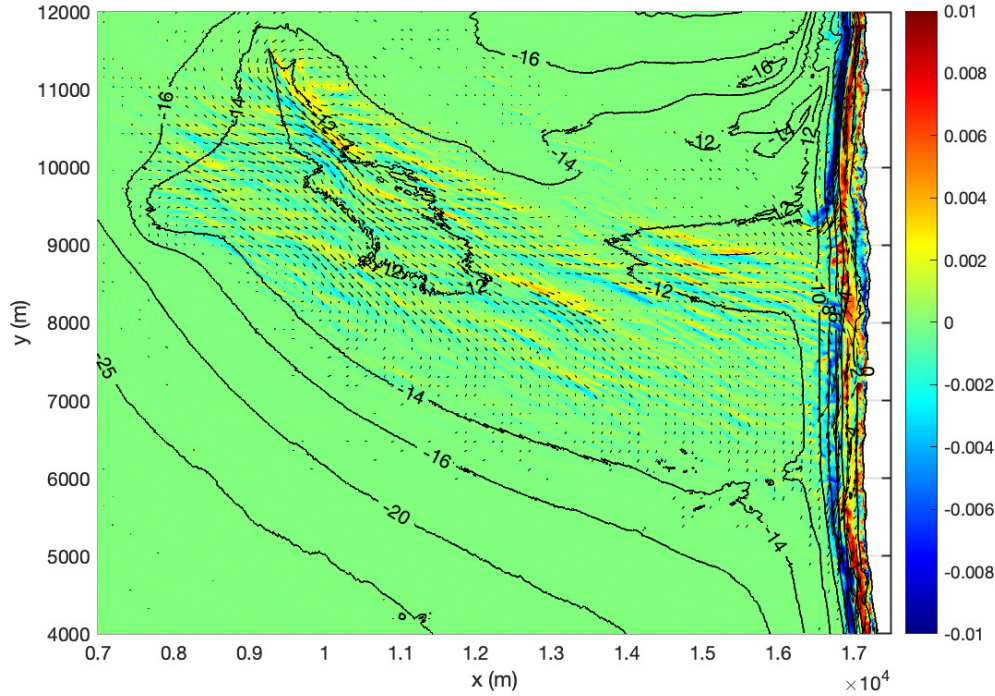


**Figure 12.** Wave height distribution (a), wave setup (b), and wave-induced circulation (c, color represents alongshore velocity) in the nearshore region denoted in the dashed line block in Figure 8. The wave-averaged quantities are analyzed over the time period of 1200 s. Case 1:  $H_{sig} = 2.49$  m,  $T_p = 11.1$  s, and  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy.

and SWAN in the simulation of the Rip Current Experiment [RCEX, *Brown et al.*, 2009; *MacMahan et al.*, 2010].

## 6 Wave-averaged processes

The previous studies showed that the nearshore circulation at Ocean Beach exhibits remarkable two-dimensional features. Although Ocean Beach has a nearly alongshore uniform bathymetry, the nearshore wave field is strongly alongshore varying because wave refraction over the ebb shoal causes wave focusing toward a narrow region at Ocean Beach. Due to the spatial variation in wave breaker height, wave-induced setup appears to have a strong alongshore nonuniformity, resulting in a dramatic change in the pressure field [*Shi et al.*, 2011; *Hansen et al.*, 2014]. The pressure gradient can be a dominant force driving nearshore circulation in the surf zone of Ocean Beach. For the time-domain Boussinesq model, wave-averaged quantities, such as wave setup, alongshore current and rip current velocities are usually obtained by wave averaging over a number of wave periods [*Chen et al.*, 2003].

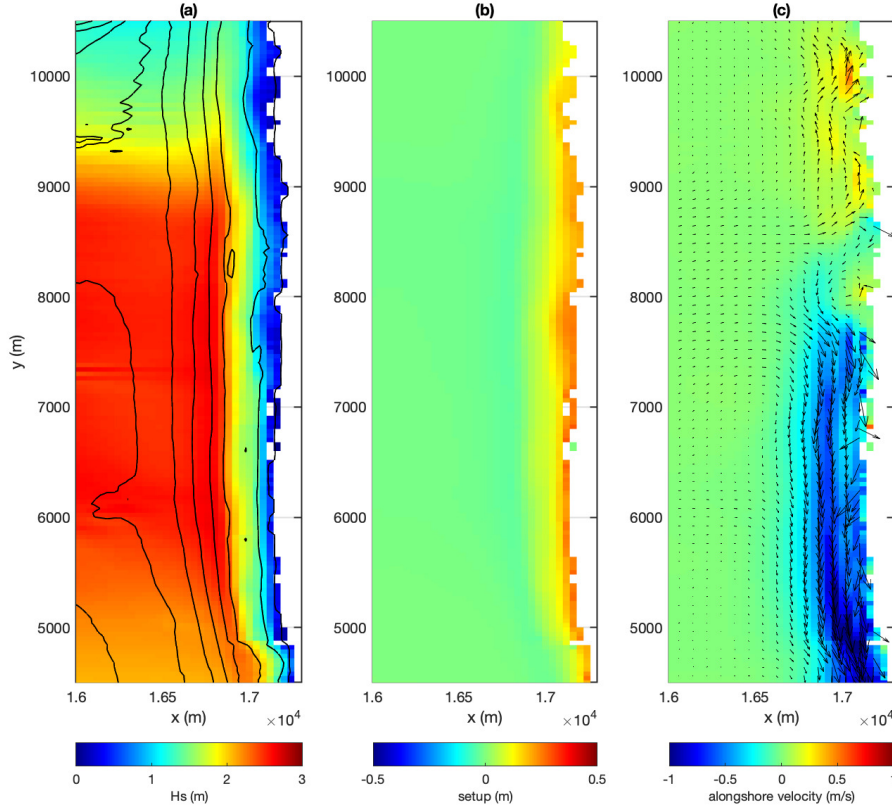


**Figure 13.** Current (arrows) and vorticity field (color) for Case 2. The wave-averaged quantities are analyzed in the time period of 1200 s. Wave conditions:  $H_{sig} = 5.1$  m,  $T_p = 14.29$  s, and  $\theta_p = 297^\circ$  measured at the CDIP 029 buoy.

## 6.1 FUNWAVE results

Figure 12 shows wave-averaged results from Case 1, including the significant wave height (a), wave setup (b), and nearshore circulation in the nearshore region denoted in the dashed line block in Figure 8. The wave-averaged quantities were calculated in a 1200 s-long time period starting from the simulation time of 1800 s to 3000 s. As shown in (a), large wave heights are basically confined in the 5000 m-long offshore region,  $y = 5000 \sim 9000$  m, with the maximum value around the center of the region. Wave height modulations caused by wave interference are in small-scale, in sizes of  $200 \sim 300$  m. Waves break in shallow water and induce wave setup in the surfzone (b). Due to the alongshore variation in offshore wave height, wave setup appears to be alongshore varying. It is interesting that the wave setup does not respond closely to the small-scale feature of the offshore wave height modulation and turns to be smoothly distributed alongshore. Figure 12 (c) shows the wave-induced nearshore circulation with arrows representing current velocity vectors and color for alongshore component velocity (blue for southward). As waves come from northwest, alongshore currents are generally southward. However, the alongshore pressure gradient associated with alongshore nonuniform wave setup induces the northward forcing, causing flow reversal, or divergence, as shown in the north region ( $8000 \sim 9500$  m). The pressure-gradient-driven process described here is consistent with the finding of *Shi et al.* [2011].

For the major storm condition simulated in Case 2, wave breaking-induced currents are more intensive compared to that in Case 1. In Fig. 13, we illustrate wave-induced current field superimposed on the vertical vorticity field, which is representative of energy dissipation due to wave breaking and bottom friction according to the vorticity conservation equation [e.g., *Chen et al.*, 1999]. The storm waves break on the ebb shoal, generating ap-



**Figure 14.** SWAN model result. Wave height distribution (a), wave setup (b), and wave-induced circulation (c, color represents alongshore velocity) in the nearshore region denoted in the dashed line block in Figure 11. Wave condition:  $H_{sig} = 2.49$  m,  $T_p = 11.1$  s, and  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy.

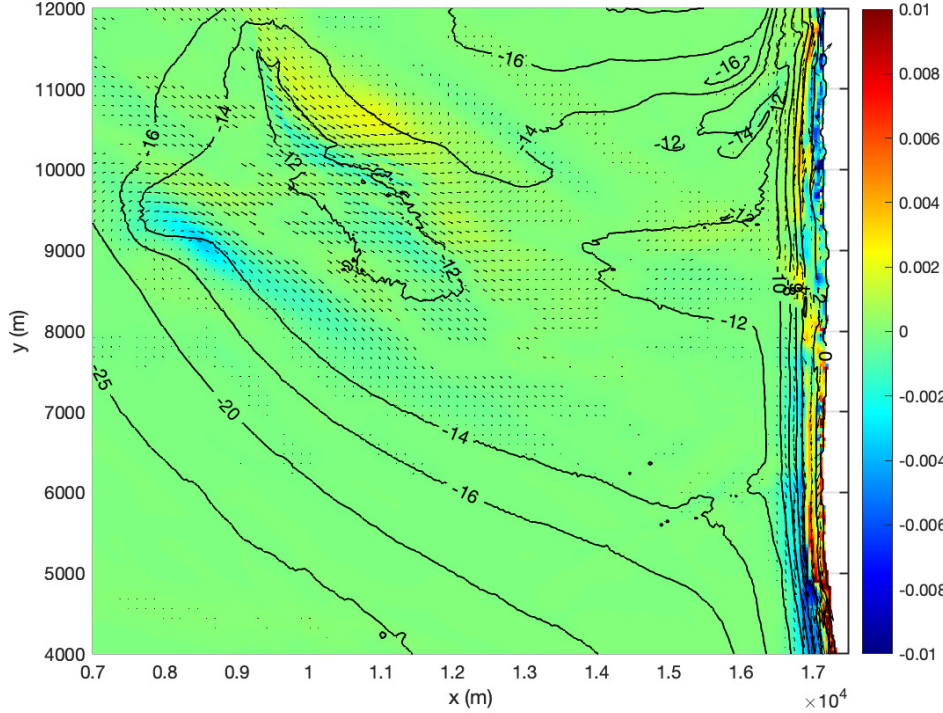
parent vortices on the shoal, which are absent in Case 1. Wave breaking induced currents on the shoal basically move in the same direction as the dominant wave direction, which can be explained by the major forcing term,  $\mathbf{k}D_w$ , where  $\mathbf{k}$  is the wavenumber vector and  $D_w$  is energy dissipation, in the vortex form of wave force formulation [Smith, 2006; Shi *et al.*, 2007]. In the nearshore region, the patterns of wave-induced current look similar to that in Case 1 but with a wider surfzone and larger current velocities caused by the overwhelming wave breaking nearshore.

## 6.2 Comparison to the wave-averaged circulation model, NearCoM

The difference in the nearshore circulation induced by the wave fields with and without the wave interference effect can be examined by the comparison between FUNWAVE and NearCoM with the same offshore wave condition. Figure 14 shows the same plot as Figure 12 but from the NearCoM model for the wave condition in Case 1. As mentioned in the last section, SWAN produced the wave focusing pattern (a), the same as in FUNWAVE, though it cannot predict the small-scale fingering feature in the wave field. Responding to the wave focusing pattern, the wave setup exhibits apparently nonuniform distribution in the along-shore direction (b) with the higher setup around the wave focusing center ( $y \sim 7,000$  m). The



southward alongshore currents are generated by the obliquely incident waves in the south region ( $y < 8500$  m). The northward reversal flows appear in the north region ( $y > 8500$  m) as a result of the pressure gradient forcing opposite to the wave forcing direction.



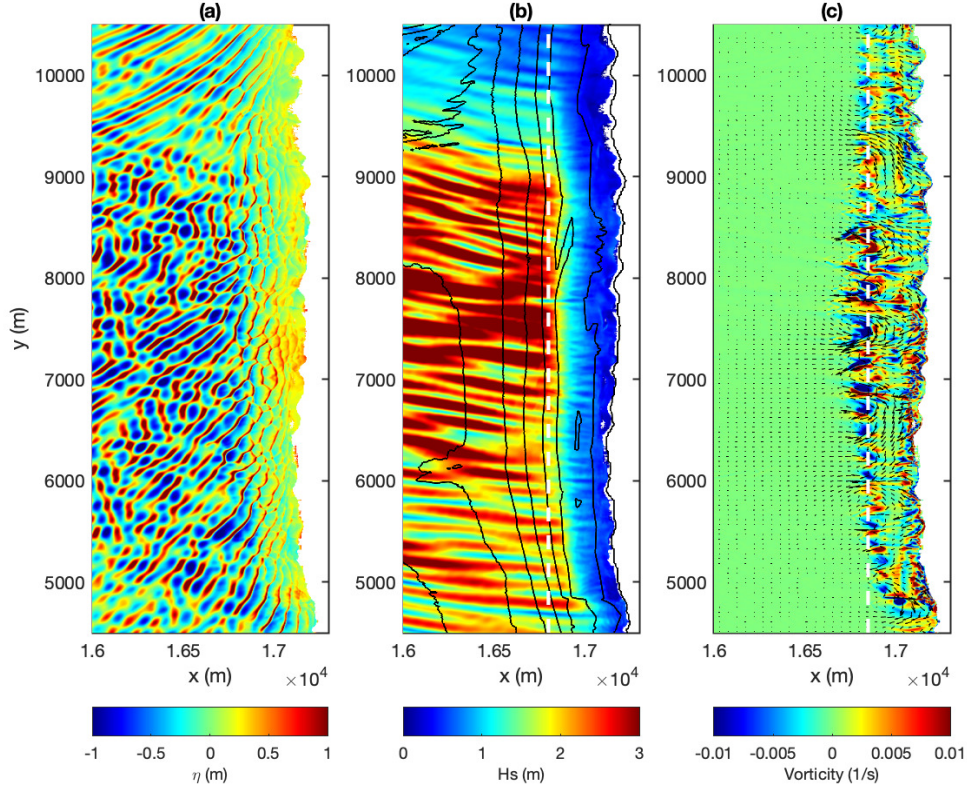
**Figure 15.** The same plot as Fig. 13 but from the NearCoM model. Case 2:  $H_{sig} = 5.1$  m,  $T_p = 14.29$  s, and  $\theta_p = 297^\circ$  measured at the CDIP 029 buoy.

For the storm scenario in Case 2, NearCoM predicted the offshore circulation on the shoal and nearshore circulation as shown in Figure 15, similar to the results from FUNWAVE. In comparison to the FUNWAVE results (Figure 13), the wave-induced offshore currents produced by NearCoM are distributed more evenly on the shoal without the small-scale strip features as shown in Figure 13. The vertical vorticity in FUNWAVE is more intense nearshore, probably due to the averaging in a short time period.

In general, the NearCoM model predicted wave-averaged processes which are similar to those predicted by FUNWAVE. However, the wave-induced circulation field predicted by NearCoM does not show much alongshore variation or small-scale structures. In addition, the wave-induced currents in NearCoM are less energetic than FUNWAVE, which will be discussed in the next section.

### 6.3 Temporal variability of wave-induced processes

Surf zone dynamics associated with wave breaking and wave-induced processes involve a wide range of temporal scales [Geiman *et al.*, 2011]. The time-domain Boussinesq model can provide a temporal variability of wave-driven nearshore circulation under the random wave forcing and wave-current interaction [Chen *et al.*, 2003]. The energetics of wave-driven current and the associated vertical vorticity field can be obtained by time-averaging of the instantaneous wave velocity field over a relatively short time period, such as 20's wave

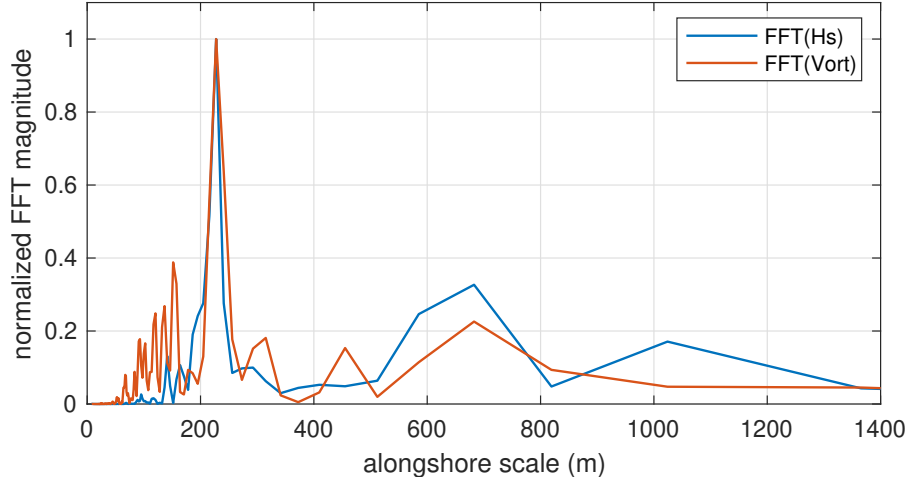


**Figure 16.** A snapshot of surface elevation (a), distributions of wave height (b) and vertical vorticity (c) in the nearshore region denoted in the dashed line block in Fig. 8. The wave-averaged quantities are processed in the time period of 1800 - 2040 s (20 peak periods). Case 1:  $H_{sig} = 2.49$  m,  $T_p = 11.1$  s, and  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy. White dashed lines are transects used for the FFT processing.

peak periods as suggested by *Chen et al.* [2003]. The vertical vorticity is generated by individual random breakers in the surfzone, producing shear waves different from those driven by a wave-averaged circulation model [*Chen et al.*, 2003]. To examine the correlation between wave interference and wave-induced circulation field, we demonstrate the vortex injection in the early stage of vorticity generation before shear waves are fully developed.

Figure 16 shows the results analyzed in the time period of 1800 s - 2040 s (20 peak periods). In the figure, a snapshot of surface elevation (a) is also presented to show wave interference patterns. The distribution of wave height (b) looks similar to that in Figure 12 (a) except the modulation patterns are more striking due to the shorter time processing versus the longer time processing (1200 s). As mentioned earlier, the nodal lines caused by wave interference are persistent and would not change much with time. Panel (c) shows the early stage of vertical vorticity generation, revealing that the injection of vortex eddies closely coincides with the fingering patterns of the offshore wave field.

To confirm the correlation between the wave modulation and vorticity generation, we applied Fast Fourier Transform (FFT) to both the alongshore distributions of wave height and vertical vorticity along the transects denoted by the white dashed lines in Figure 16 (b) and (c). Fig. 17 compares the amplitudes of FFT for wave height and vorticity modulations ver-



**Figure 17.** Amplitudes of the Fast Fourier Transform for alongshore wave modulation (blue) and vorticity modulation (red) along the transects denoted by the white dashed lines in Figure 16 (b) and (c). The amplitudes were normalized by the corresponding maximum FFT values.

350 sus alongshore scales. The amplitudes in the comparison were normalized by their maximum  
 351 values of FFT. For the wave height modulation (blue), the peak of the FFT amplitude ap-  
 352 pears at 220 m, meaning that the alongshore scale of the nodal lines is around 220 m. As ex-  
 353 pected, the peak for the vorticity field (red) occurs around the same place, indicating the ma-  
 354 jor length scale for vortices in the early stage is consistent with the scale of the nodal lines.  
 355 The vorticity generation is closely correlated to the wave interference caused wave height  
 356 modulation.

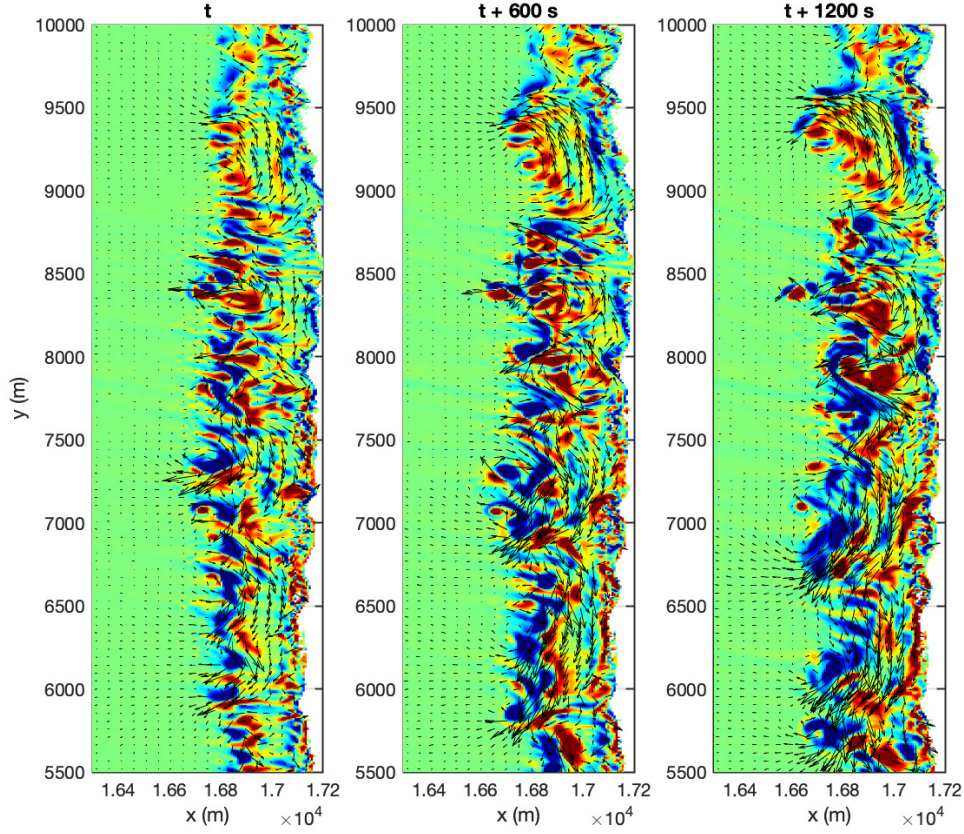
357 The energetic and transport features of vertical vorticity can be found in the snapshots  
 358 of the vorticity field in Figure 18, which are taken 600 s apart, starting from  $t = 2040$  s. The  
 359 vertical vorticity in each snapshot was calculated by averaging over the same short time pe-  
 360 riod (240 s) as the case above. The figure shows that the instantaneous vorticity is advected  
 361 by alongshore currents (refer to Figure 12) southward in the south region ( $y < 8000$  m) and  
 362 northward in the north region. Small-scale rip current cells appear all over the place inside  
 363 the surf zone.

## 364 7 Conclusions

365 The study was motivated by the finding in *Smit et al.* [2016] who observed the out-  
 366 standing wave interference phenomena at the ebb-tidal delta of San Francisco, CA. Our  
 367 curiosity is how the wave interference affects the nearshore wave field and wave-induced  
 368 nearshore circulation, which has been studied previously using wave-averaged circulation  
 369 models. In this study, we used the time-domain Boussinesq wave model, FUNWAVE-TVD,  
 370 to investigate waves and wave-induced circulation processes with emphasizing wave inter-  
 371 ference caused by the ebb shoal and its effects on nearshore circulation. The model results were  
 372 compared with the wave-averaged model, NearCoM, used in a previous study.

373 The Boussinesq model predicted the wave interference phenomena caused by the ebb  
 374 shoal, with interference scales of 150 ~ 310 m, consistent with the Radar observation by *Smit*  
 375 *et al.* [2016]. The model shows the small-scale fingering structures in the wave height dis-  
 376 tribution resulting from wave interference, which are persistent with nodal lines unchanged  
 377 with time. The wave height modulation can reach up to 50% of the wave height predicted by  
 378 the conventional phase-averaged wave model. The field observation offshore of Ocean Beach





**Figure 18.** Time sequence of current and vorticity field averaged over 240 s. Case 1:  $H_{sig} = 2.49$  m,  $T_p = 11.1$  s, and  $\theta_p = 290^\circ$  measured at the CDIP 029 buoy.

in the previous studies did not resolve the wave field modulation due to the sparse deployments of instruments.

The impact of wave interference on nearshore circulation is significant. The modulated wave field induces small-scale flow structures in nearshore circulation, which were not predicted by the conventional wave-averaged circulation model. However, the small-scale modulation in the wave field seems not to generate alongshore variation in wave setup in such scales. Therefore, in a large-scale view, the alongshore currents predicted by the Boussinesq model still keep the major features shown in the wave-averaged circulation model, such as the flow divergence caused by the pressure gradient force associated with the wave setup. For a storm wave condition, waves break on the ebb shoal, inducing shoreward-directed currents in the breaking region, indicated in both the Boussinesq and the wave-averaged circulation model. The Boussinesq model shows smaller-scale stripe features of breaking wave-induced current and vorticity fields versus the wave-averaged circulation model in which the current and vorticity are evenly distributed on the shoal.

The time-domain Boussinesq model predicted the temporal variability of wave-induced processes. The wave field modulation is highly correlated to the vorticity generation nearshore. The alongshore varying wave breakers tied with wave interference patterns are sources of the vorticity generation and cause energetic vortex eddies in the surf zone.



The nearshore circulation at Ocean Beach is affected by tides and tidal currents, especially in the nearshore area close to the inlet as discussed in Shi *et al.* [2011] and Hansen *et al.* [2013]. Due to a large computational cost, we did not include tides in the time-domain Boussinesq model. However, the model reveals more complete physical processes associated with wave interference effects and provides a promising prospect for the future research on wave-induced processes in the large ebb shoal-beach system.

**Data Availability Statement** The post-processed numerical results and measurement data used in this research are archived at (<http://doi.org/10.5281/zenodo.6336156>).

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