

1 **Title:**

2 Simulating Spatial and Temporal Dynamics of Soil Moisture and Gully Flow Using  
3 Improved Grid-Xinanjiang Model with a Prior Parameter Estimates

4 **Running head:**

5 Spatial and Temporal Dynamics of Soil Moisture and Gully Flow

6 **Key words:**

7 Gullies; Improved Grid-Xinanjiang model; Priori parameters estimation; Global  
8 Digital Soil Mapping System; Tunxi watershed

9 **Authors' name:**

10 Bingxing Tong<sup>1</sup>, Zhijia Li<sup>1</sup>, Cheng Yao<sup>1\*</sup>, Moyang Liu<sup>2</sup>, Junfu Gong<sup>1</sup>

11 **Corresponding Author:**

12 Cheng Yao, yaocheng@hhu.edu.cn

13 **Authors' institutions:**

14 <sup>1</sup> College of Hydrology and Water Resources, Hohai University, Nanjing, 210098,  
15 China

16 <sup>2</sup> Fenner School of Environment & Society, Australian National University, Canberra,  
17 ACT 0200, Australia

18 **Acknowledgments:**

19 This study was supported by the National Natural Science Foundation of China  
20 (Grant No. 52079035), the Fundamental Research Funds for the Central Universities  
21 (Grant No. B200203052), and the Postgraduate Research & Practice Innovation  
22 Program of Jiangsu Province (KYCX20\_0466).

23

24 **Simulating Spatial and Temporal Dynamics of Soil Moisture and**  
25 **Gully Flow Using Improved Grid-Xinanjiang Model with a Prior**  
26 **Parameter Estimates**

27 **Abstract:** To systematically generalize the influence of gullies on floods, a distributed

28 model named Improved Grid-Xinanjiang (GXAJ), and a priori parameters estimation  
29 scheme based on the Global Digital Soil Mapping System (SoilGrids) are proposed.  
30 Within a watershed divided into a series of orthogonal cells, shallow furrows and  
31 trenches inside the cells are conceptualized as primary gullies, in which water  
32 movement is simulated by kinematic wave equation considering the gullies density,  
33 and well-developed grooves between cells are considered as main gullies, where  
34 water moves as a kinematic wave and enters the rivers. The simulation of 27 flood  
35 events in the Tunxi watershed of Anhui Province from 2008 to 2017 was  
36 implemented, and the simulation results were compared with that of Xinanjiang  
37 model (XAJ). The relative runoff volume error and flood peak error of the GXAJ  
38 model and XAJ model are 8.4% and 10.7%, 8.9% and 12.1%, respectively. The GXAJ  
39 model outperforms in the simulation of flood peak, and is capable of producing the  
40 dynamics of soil water and gullies flow. The spatial characteristics and the sensitivity  
41 of parameters, free storage capacity and gullies density, at various phases, that is,  
42 initial, rise, peak, fall and tail, have been analyzed. The value of free storage capacity  
43 decreases and then increases with the increase of altitude and distance from the river.  
44 The gullies density in the bank and ridge area is greater than that in the middle of the  
45 slope segment. Sensitivity analysis shows that gullies density has the noticeable  
46 influence on the relative runoff volume error and Nash-Sutcliffe coefficient in the rise  
47 phase, while free water storage capacity has a significant effect on the relative runoff  
48 volume error during the flood rise phase and Nash-Sutcliffe coefficient in peak phase,  
49 respectively.

50 **Keywords:** Gullies; Improved Grid-Xinanjiang model; Priori parameters estimation;  
51 Global Digital Soil Mapping System; Tunxi watershed

52

## 53 **1 INTRODUCTION**

54 The gullies have a significant impact on the rainfall-runoff process of watersheds  
55 (Lai, Chen, Wang, Yu & Bai, 2020; Kirkby, 1978). According to the development  
56 degree from low to high, the gullies on the slope can be generally classified as

57 shallow furrows, trenches and well-developed grooves. By the gullies, the runoff on  
58 the hillside may converge and move quickly, forming a steeply rising flash flood that  
59 threatens the lives and property of people living near the river (McClellan, Dawson, &  
60 Kilsby, 2020; Liang, Li, Yuan, & Liao, 2019). Gullies are shaped by the interaction  
61 between hydrodynamic factors and soils during slope erosion. The hydraulic  
62 characteristics of a river are influenced by the gullies morphology, which in turn  
63 affects the erodibility and stability of the soil. The erosive power of the flow increases  
64 continuously with the convergence of the sheet flow on slope surface. When the  
65 erosive power exceeds the stabilizing capacity of the soil surface particles, soil  
66 erosion occurs and primary shallow furrows are formed, which interconnect and  
67 contribute to the creation of higher level gullies (Garbrecht, & Martz, 1997; Wang,  
68 Xu, Wang, & Wu, 2020; Martz, & Garbrecht, 1995).

69 However, it is difficult to systematically quantify and generalize the gullies'  
70 ability to regulate flow compared to the river network. On the one hand, despite the  
71 implementation of field surveys and remote sensing image classification techniques, it  
72 is almost impossible for gullies to be accurately identified (Zeng, Xu, & Wan, 2020;  
73 Thielen, Lücke, Diekkrüger, & Richter, 1999). On the other hand, influenced by the  
74 diverse topography and variation of rainfall, the flow in the gullies changes rapidly  
75 during the rainfall-runoff process, making it difficult to collect sufficient observation  
76 records (Zhang et al, 2019; Montgomery, 2010). Therefore, there are several  
77 confluence methods for simulating river flow movement, but few studies on water  
78 flow in slope gullies (Fraga, Cea, & Puertas, 2019; Daniel, Guo, & John, 2020).

79 Simulation of rainfall-runoff process, a subject that has been intensively studied  
80 in the last decades, starts from the adoption of empirical methods (Xia, Wang, Gan,  
81 2019; Kuo et al, 1999). The second stage is mainly characterized by the application of  
82 conceptual hydrological models after the 1970s, such as the Sacramento model (SAC)  
83 (Sorooshian, Duan, & Gupta, 1993; Gupta, Sorooshian, & Yapo, 1999; Najafi,  
84 Moradkhani, & Jung, 2011; Gupta, Wagener, & Liu, 2008). In the third stage since the  
85 21st century, the application of distributed hydrological models such as the remote

86 sensing-based WRF-Hydro model, and coupled atmospheric-hydrological modeling  
87 systems have further contributed to the development of flood simulation and  
88 prediction (Koren, Reed, & Smith, 2004; Reed, 2004; Ajami, Newsha, Duan, Gao,  
89 Sorooshian, & Soroosh, 2006; Abbaszadeh, Gavahi, & Moradkhani, 2020 ). In August  
90 2016, the WRF-Hydro-core National Water Model has been used to support  
91 distributed runoff simulations for approximately  $2.7 \times 10^6$  rivers across the United  
92 States at a resolution of 250 meters per hour (Xiang, Vivoni, Gochis, & Mascaro,  
93 2017; Zhang, Lin, Gao, & Fang, 2020; Lahmers, Gupta, Castro, Gochis, &  
94 Hazenberg, 2019; Viterbo, Mahoney, Read, Salas, & Cifelli, 2020; HaNSCn, Shiva,  
95 Mcdonald, & Nabors, 2019). In China, the Xinanjiang model (XAJ) proposed by  
96 Zhao Renjun in the 1970s was widely adopted (Zhao, 1984; Wang, & Zhao, 1989; Li,  
97 Liang, Kan & Zhang, 2016). Based on the statistical analysis of the rainfall-runoff  
98 relationship in the Xinanjiang watershed, the saturated-excess runoff mechanism was  
99 adopted in the XAJ model to achieve reasonable floods simulation. The Grid-  
100 Xinanjiang model (GXM), that is, a distributed version of the well-known XAJ model  
101 has also been proposed based on discrete geophysical data (Yao, Li, Bao, & Yu, 2009;  
102 Bao, et al, 2011; Li, et al, 2017). However, the effect of hillside gullies on flood has  
103 not been addressed in the GXM, especially in mountainous areas of China, where  
104 flash flood simulations are limited by poor topographic measurements and  
105 hydrological observations (Liang, Lu, Chen, Liu & Lin, 2020; Yao, Li, Yu, & Zhang,  
106 2012).

107 Therefore, reasonably quantifying the influence of gullies on floods based on  
108 topographic characteristics of study area, developing appropriate model structure and  
109 algorithm, and realizing the prior estimation of spatial parameters that can support  
110 operational flood simulation have become critical issues discussed in this study. Based  
111 on the systematic generalization of hillside gullies, an improved distributed model  
112 named Improved Grid-Xinanjiang (GXAJ). Limited by the data observation level at  
113 that time, the GXM model was mainly based on the approximate soil and vegetation  
114 classification and the empirical table recording the corresponding hydrological

115 property values for each classification to estimate the main parameters such as free  
116 water storage capacity and filed capacity. Although this approach can meet the driving  
117 requirements of distributed models, it is difficult to achieve a fine-grained quantitative  
118 description of the spatial characteristics of parameters. Further, a priori parameters  
119 estimation scheme based on the Global Digital Soil Mapping System (SoilGrids)  
120 (Sun, Wang, Hui, Jing, & Feng, 2020; Grunwald, Thompson, & Boettinger, 2011;  
121 Tomislav et al, 2017) are proposed in this study. Within a watershed divided into a  
122 series of orthogonal cells, shallow furrows and trenches inside the cells are  
123 conceptualized as primary gullies, in which water movement is simulated by  
124 kinematic wave equation considering the gullies density, and well-developed gullies  
125 between cells are considered as main gullies, where water moves as a kinematic wave  
126 and enters the rivers. The simulation of 27 flood events in the Tunxi watershed of  
127 Anhui Province from 2008 to 2017 was implemented, and the simulation results were  
128 compared with that of Xinanjiang model (XAJ) and measurement. Quantitative  
129 analyses of the sensitivity and spatial characteristics of the parameters, free water  
130 storage capacity and gullies density are emphasized. In addition, the dynamics of  
131 watershed-scale free water content and gullies flow during rainfall-runoff process are  
132 presented.

133

## 134 **2 THE IMPROVED GRID-XINANJIANG MODEL**

135 The Grid-Xinanjiang model (GXM), that is, a distributed version of the well-  
136 known XAJ model, has also been proposed by Yao etc. In GXM model, the watershed  
137 is discretized into a series of orthogonal cells where runoff generation using saturated-  
138 excess mechanism are implemented, the flow direction of each cell is identified with  
139 the Digital Elevation Model (DEM) to obtain the confluence sequence of the runoff. A  
140 series of cases where water balance has been achieved demonstrated the stability of  
141 the distributed structure, therefore it was retained in the upgrade of the Grid-  
142 Xinanjiang model. Furthermore, the effect of gullies on floods has been  
143 systematically generalized contributing to Improved Grid-Xinanjiang (GXAJ)

144 developed in this study. Specifically, the gullies have been generalized into shallow  
145 furrows, trenches and well-developed grooves according to the level of development.  
146 The shallow furrows and trenches inside cells are conceptualized as primary gullies,  
147 and the well-developed grooves between cells are considered as the main gullies. The  
148 runoff generated in the cell would first enters the primary gullies using the kinetic  
149 wave equation considering the gullies density, and then flows through multiple cells  
150 in the main gullies. Finally, gullies flow enters river and reaches the outlet of  
151 watershed after the Muskingum-Cunge confluence evolution. The rainfall-runoff  
152 process in the slope cells is divided into four parts: the saturation-excess runoff  
153 generation, the runoff flows into the primary gullies on the slope, flow movement in  
154 primary gullies and flow movement through multiple cells in main gullies. Gullies  
155 flow from cells near bank would be discharged into river, then participate in river  
156 routing to reaches the outlet of watershed, forming flood hydrograph (Figure 1).

157

## 158 **2.1 Runoff generation and overland flow within cells**

159 With the triplex evaporation and saturation-excess runoff mechanism, process of  
160 evaporation, dynamic change of soil moisture and runoff generation in each cell has  
161 been simulated. In evaporation and runoff generation, the soil is stratified into three  
162 layers named upper, lower and bottom. Water in soil is divided into free water and  
163 tension water depending on whether it can flow freely by gravity or not. The rainfall  
164 would first infiltrate into the soil to meet the tension water deficiency. After the  
165 tension reservoir is full, water would flow out from the side and bottom of the upper  
166 soil layer, respectively, and turn into interflow and groundwater. When the upper layer  
167 of soil moisture has been saturated, the excess water flows over the sloping surface as  
168 overland runoff. If the time required for various runoff to flow through the slope  
169 surface exceeds the time step used in the model, only a portion of the generated runoff  
170 is able to enter gullies and further participate in the convergence routing. It can be  
171 argued that the slope surface has a moderating effect, which is quantified in this study  
172 by the linear reservoir technique.

$$R_1 = R_0 \times \gamma + InR \times (1 - \gamma) \quad (1)$$

173 where  $R_1$  is the runoff entering gullies at present period,  $R_0$  is the outflow from the soil  
 174 to gullies in the last period,  $InR$  is the runoff generated in present period,  $\gamma$  is the  
 175 coefficient of linear reservoir.

176

## 177 2.2 Flow movement in primary gullies

178 Shallow furrows and trenches, the important paths for flow within cells, are  
 179 generalized as primary gullies, in which the water movement is modeled by kinematic  
 180 wave equation considering the gullies density ( $D$ ).

$$\frac{\partial A}{\partial t} + \frac{1}{D} \frac{\partial Q}{\partial x} = 0 \quad (2)$$

$$S_f = S_o \quad (3)$$

181 where  $A$  is the cross section area,  $Q$  is the discharge in primary gullies, and  $t$  and  $x$   
 182 refer to the time and space items, respectively.  $S_f$  is the hydraulic gradient,  $S_o$  is the  
 183 bottom slope of primary gullies.

184 In combination with the kinematic wave equation and the hydraulic  
 185 characteristics of the main gullies (e.g. wetted perimeter, roughness, etc.), the  
 186 differential format is used to perform numerical calculations for water flow  
 187 simulation.

$$\sigma \beta Q^{\beta-1} \frac{\partial Q}{\partial t} + \frac{1}{D} \frac{\partial Q}{\partial x} = 0 \quad (4)$$

$$A = \sigma Q^\beta, \sigma = \left[ \frac{n P^{2/3}}{\sqrt{S_f}} \right]^{3/5}, \beta = 3/5 \quad (5)$$

$$Q_{i+1}^{j+1} = \frac{\left[ \sigma \beta \left( \frac{Q_{i+1}^j + Q_i^{j+1}}{2} \right)^{\beta-1} Q_{i+1}^j + \frac{\Delta t}{D \Delta x} Q_i^{j+1} \right]}{\left[ \sigma \beta \left( \frac{Q_{i+1}^j + Q_i^{j+1}}{2} \right)^{\beta-1} + \frac{\Delta t}{D \Delta x} \right]} \quad (6)$$

188 where  $P$  is the wetted perimeter,  $n$  is the roughness,  $\sigma$  and  $\beta$  are the coefficients of  
 189 equation,  $i$  and  $j$  are the space and time index, respectively, and  $\Delta x$  and  $\Delta t$  are the  
 190 space and time step, respectively.

191

### 192 2.3 Flow movement in main gullies and water exchange mechanism between cells

193 The well-developed grooves are used as the main gullies to achieve the flow of  
194 water between cells. The motion of the water flow in the main gullies can be  
195 numerically simulated using the kinematic wave equation of motion combined with  
196 Manning's formula.

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = q \quad (7)$$

$$S_f = S_g \quad (8)$$

$$Q_{i+1}^{j+1} = \frac{\left[ \alpha\beta \left( \frac{Q_{i+1}^j + Q_i^{j+1}}{2} \right)^{\beta-1} Q_{i+1}^j + \frac{\Delta t}{\Delta x} Q_i^{j+1} + \frac{q_{i+1}^j + q_i^{j+1}}{2} \right]}{\left[ \alpha\beta \left( \frac{Q_{i+1}^j + Q_i^{j+1}}{2} \right)^{\beta-1} + \frac{\Delta t}{\Delta x} \right]} \quad (9)$$

197 where  $S_g$  is the slope of the main gully, and  $q$  is the lateral inflow.

198 When simulating the water movement, the possible hydraulic connection between  
199 adjacent cells should be considered. In this study, after the cell receives the incoming  
200 water from the uphill section, the outflow is generated only when the tension water  
201 deficit is satisfied.

202

### 203 2.4 River flood routing

204 Compared with gullies, water flow movement in rivers is more stable. To reflect  
205 the influence of geographical factors such as section width, slope, and roughness on  
206 water flow, the Muskingum-Cunge method has been utilized for water movement  
207 simulation in the river.

$$Q_{out,t} = C_0 \times Q_{i,t-1} + C_1 \times Q_{i,t} + C_2 \times Q_{out,t-1} \quad (10)$$

$$X = 0.5 \left( 1 - \frac{Q}{BS_f C_k \Delta x} \right), \quad K = \frac{\Delta x}{C}, \quad C = \frac{C_k \Delta t}{\Delta x} \quad (11)$$

$$C_0 = \frac{KX + 0.5 \Delta t}{K(1-X) + 0.5 \Delta t}, \quad C_1 = \frac{0.5 \Delta t - KX}{K(1-X) + 0.5 \Delta t}, \quad C_2 = \frac{K(1-X) - 0.5 \Delta t}{K(1-X) + 0.5 \Delta t} \quad (12)$$

208 where:  $C_0$ ,  $C_1$  and  $C_2$  are formula coefficients,  $Q_{out,t}$  is the outflow at present period,

209  $Q_{in,t-1}$ , is the inflow at the previous period,  $Q_{in,t}$  is the inflow at present period, and  $Q_{out,t-1}$   
210 is the outflow at the previous period,  $X$  is the coefficient of Muskingum-Cunge  
211 method, being inverse correlate with the downstream congestion on the upstream  
212 flow,  $K$  is the slope of the storage-discharge relationship,  $B$  is the width of the river  
213 cross,  $C_k$  is the velocity of flood wave, and  $C$  is the average flow velocity.

214

### 215 **3 PARAMETERIZATION SCHEME**

216 The main parameters of GXAJ model are shown in Table 1:

217 Free and tension water storage capacities indicated by SM and WM, respectively,  
218 are key parameters for runoff generation in the saturation-excess mechanism. Flow  
219 generally occurs only after the moisture exceeds the field capacity of soil, so the  
220 amount of water that the soil could hold between the field capacity and the wilting  
221 point (the lowest possible moisture content of the soil in natural conditions) is the  
222 tension water storage capacities WM. The generation of surface runoff means that the  
223 soil is saturated, and the amount of water needed to saturate the soil from its field  
224 capacity is the free water storage capacity SM. Parameters KI and KG are used to  
225 classify runoff components, representing the ratio of the water flowing from the soil  
226 sides and the bottom to the amount of water that can flow freely by gravity in the soil  
227 during the calculation period, respectively. CI and CG are the coefficient of linear  
228 reservoir mentioned in section 2.1 for interflow and groundwater, respectively.  
229 Gullies density describes the spatial characteristics of the gullies and indicating the  
230 ratio of the total length of gullies to the area of the watershed. Manning roughness,  
231 expressed as Mn, reflects a comprehensive dimensionless number that affects the  
232 resistance to water flow.

233 A priori estimation scheme named GeoPara (Figure 2) was proposed for the  
234 spatial parameters of the GXAJ model by combining the soil data provided by the  
235 Global Digital Soil Mapping System (SoilGrids). Soil hydrological properties  
236 including saturated capacity ( $\theta_s$ ), field capacity ( $\theta_f$ ), wilting point ( $\theta_{wp}$ ) and saturated  
237 hydraulic conductivity (KS) have been obtained based on the characteristic of

238 underlying surface such as soil texture, and then these properties were utilized to  
239 estimate the WM, SM, KI, KG, CI and CG of the GXAJ model. Furthermore,  
240 gradient, width and Manning roughness have been extracted from the Digital  
241 Elevation Model for flow movement simulation in channel system consisted of rivers  
242 and gullies (Mohammad & Seyed, 2019; Diaz, 2005). For gullies density D, it can be  
243 estimated by combining the distance to the gullies and the infiltration capacity of the  
244 soil based on Langbein's topographic survey (Langbein, 1947) and Horton's  
245 theoretical analysis (Horton, 1936).

246

### 247 **3.1 Hydrological properties of soil**

248 The change range of soil moisture gradually decreased in the deeper layer,  
249 according to which the soil can be roughly divided into active and stable layers from  
250 top to bottom (Wang, Fu, Zhang, & Xu, 2019). Soil active layer plays an important  
251 role in rainfall-runoff processes, but are difficult to accurately identify and measure at  
252 the watershed scale (Levia, & Frost, 2003; Haney, SeNSCman, Hons, & Zuberer,  
253 2000). Fortunately, water is one of the most active variables in nature and it influences  
254 the evolution and fertility of the soil, so that, in general, the organic matter content of  
255 the active layer is higher than at the bottom of the soil (Saxton, & Rawls, 2006; Kim,  
256 2017), which could provide a reference for soil stratification.

257 Based on the content of organic matter at the depth of 5, 15, 30, 60, 100 and 200  
258 cm from the surface provided by SoilGrids, the change trend could be obtained that  
259 would contribute to the vertical stratification of the soil and the estimation of the  
260 thickness of the spatially active layer (Figure 3).

$$L_h = T_{Min} + (T_{Max} - T_{Min}) \times \left( \frac{L_a}{T_M} \right) \quad (13)$$

261 where  $L_h$  is the thickness of the active soil layer.  $T_{min}$  and  $T_{max}$  are the minimum and  
262 maximum thickness of the active soil layer, respectively, which can be estimated by  
263 the ratio of organic matter content to total organic matter within a given soil layer. As  
264 shown in Figure 2,  $\alpha$  and  $\beta$  indicate two given layers. The values of  $\alpha$  and  $\beta$  could

265 be obtained from the field survey and operational experience of underlying surface  
 266 characteristics generalization in rainfall-runoff process simulation in the watershed.  $L_a$   
 267 is the thickness of the soil aeration zone, and  $T_M$  is maximum soil thickness of  
 268 watershed, referring to SoilGrids.

269 Using experimental formula given by USDA-ARS Hydrology and Remote  
 270 Sensing Laboratory (Saxton, & Rawls, 2006), the hydrological characteristics of soil  
 271 including saturated capacity ( $\theta_s$ ), field capacity ( $\theta_f$ ), wilting point ( $\theta_{wp}$ ) and saturated  
 272 hydraulic conductivity (KS) have been estimated based on soil texture data (silt, clay,  
 273 sand and organic matter content) provided by SoilGrids.

$$\left\{ \begin{array}{l} \theta_y = -0.024 \times Ratio_s + 0.487 \times Ratio_c + 0.006 \times Ratio_{Om} + 0.005 \times Ratio_s \times Ratio_{Om} \\ \quad - 0.013 \times Ratio_c \times Ratio_{Om} + 0.068 \times Ratio_s \times Ratio_c + 0.031 \\ \quad \theta_\phi = \theta_y + (0.14 \times \theta_y - 0.02) \end{array} \right. \quad (14)$$

$$\left\{ \begin{array}{l} \theta_\epsilon = 0.278 \times Ratio_s + 0.034 \times Ratio_c + 0.022 \times Ratio_{Om} - 0.018 \times S \times Ratio_{Om} \\ \quad - 0.027 \times Ratio_c \times Ratio_{Om} - 0.584 \times Ratio_s \times Ratio_c + 0.078 \\ \quad \theta_\sigma = \theta_\epsilon + (0.636 \times \theta_\epsilon - 0.107) \end{array} \right. \quad (15)$$

$$\left\{ \begin{array}{l} \theta_\mu = -0.251 \times Ratio_s + 0.195 \times Ratio_c + 0.011 \times Ratio_{Om} + 0.006 \times Ratio_s \times Ratio_{Om} \\ \quad - 0.027 \times Ratio_c \times om + 0.452 \times Ratio_s \times Ratio_c + 0.299 \\ \quad \theta_\tau = \theta_\mu + 1.283 \times \theta_\mu^2 - 0.374 \times \theta_\mu - 0.015 \\ \quad \theta_s = \theta_\tau + \theta_\sigma - 0.097 \times Ratio_s + 0.043 \end{array} \right. \quad (16)$$

$$\left\{ \begin{array}{l} \theta_f = 1 - ((1 - \theta_s) \times 1.2) \\ R = (\log \theta_\tau - \log \theta_y) / (\log 1500 - \log 33) \\ Ks = 1930 \times (\theta_s - \theta_\tau)^{(3-R)} \end{array} \right. \quad (17)$$

274 where  $Ratio_s$ ,  $Ratio_c$  and  $Ratio_{om}$  are sand, clay and organic matter content in weight,  
 275 respectively;  $\theta_y$ ,  $\theta_\epsilon$ ,  $\theta_\sigma$ ,  $\theta_\mu$ ,  $\theta_\tau$  and R are intermediate variables.

276

### 277 3.2 Spatial parameters

278 In addition to the hydrological characteristics of the soil, the parameters SM and  
 279 WM, which quantify the water storage capacity, are also related to the layer thickness  
 280 (Equation 18). It should be noted that due to the stratification of the soil, the value of  
 281 the tension water storage capacity of each soil layer also needs to be considered

282 separately. According to saturation-excess runoff mechanism, the value of the  
 283 parameter WM is the sum of the tension water storage capacity of the upper soil layer  
 284 (WUM), i.e. the active layer in this study, and the rest of the soil layers, including the  
 285 lower (WLM) and deep layers (WDM). The ratio of WUM to WM could be estimated  
 286 by the ratio of active layer to soil vadose zone. WDM that has tiny impact on hourly  
 287 floods could be distinguished from WM by operation experience, of which the value  
 288 is about 40 to 50% of WM, generally. Parameters KI and KG, gullies capacity of soil  
 289 water, which are affected by terrain slope (Equation 19). CI and CG are the  
 290 quantization of runoff regression, which are closely related to the length and gradient  
 291 of slope segments (Equation 20).

$$\begin{cases} SM = L_h \times (\theta_s - \theta_f) \\ WM = L_a \times (\theta_f - \theta_{\phi}) \end{cases} \quad (18)$$

$$\begin{cases} KI = \frac{2 \times KS_u \times S_{oc}}{(\theta_s - \theta_f) \times L_{hill}} \\ KG = \frac{2 \times KS_m \times S_{oc}}{(\theta_s - \theta_f) \times L_{hill}} \end{cases} \quad (19)$$

$$\begin{cases} CI = e^{(-1/tthi)} \\ CG = e^{(-1/tthg)} \end{cases} \quad (20)$$

292 where  $S_{oc}$  is the terrain slope,  $KS_u$  and  $KS_m$  are the saturated hydraulic conductivity of  
 293 upper and lower layer of soil,  $L_{hill}$  is the length of slope segments,  $tthi$  and  $tthg$  are  
 294 time required for interflow and groundwater flow through slope segments, estimated

295 on the basis of  $L_{hill}$ ,  $S_{oc}$ ,  $KS_u$  and  $KS_m$  ( $tthi = \frac{L_{hill}}{KS_u \times slp}$ ,  $tthg = \frac{L_{hill}}{KS_m \times slp}$ ).

296

### 297 3.3 River width

298 The upstream gullies area and slope origin moment, the two main factors of the  
 299 proposed river width model (GeoRW), are used to quantify the trend and terrain factor  
 300 for describing the variation of river width from upstream to downstream. The trend

301 factor  $RW_a$  and terrain factor  $RW_s$  are expressed on the basis of upstream gullies area  
 302 and slope origin moment, respectively.

$$B = \delta \times RW_{te} + \mu \quad (21)$$

$$RW_{te} = RW_a \times RW_s \quad (22)$$

$$\left\{ \begin{array}{l} RW_a = 1 - \frac{\sqrt{MaxH} - \sqrt{H}}{\sqrt{MaxH} - \sqrt{T}} \\ RW_s = 1 - \frac{S_{mean}}{MaxS} \end{array} \right. \quad (23)$$

303 where B is the river width,  $\delta$  is the proportional coefficient of the river width model,  
 304  $\mu$  is the basic river width,  $RW_s$  is the slope factor,  $RW_a$  is the confluence  
 305 accumulation factor, MaxH is the maximum value of the cumulative confluence, that  
 306 is, the cumulative confluence at the outlet of the watershed, H is the cumulative  
 307 confluence in the river cell, T is the cumulative confluence threshold when the river  
 308 network is extracted, and  $S_{mean}$  is centered on the river cell unit, taking  $(B_{max} \times 1.414)/2$   
 309 as the first-order origin distance of the slope within the radius,  $B_{max}$  is the width of the  
 310 widest channel section. MaxS is the maximum value of the first-order origin distance  
 311 of the slope.  $\delta$  and  $\mu$  can be estimated from a small number of sample river sections  
 312 in satellite images (Tong, Li, Wang, Yao, & He, 2020; Horritt, & Bates, 2001). The  
 313 bottom slope of the river section is obtained from the ratio between the elevation  
 314 difference between the upper and lower reaches and the length of the river section,  
 315 and the slope is approximated by  $S_{mean}$ .

316

### 317 **3.4 Gullies density**

318 In 1939 and 1940, Langbein and his colleagues, have conducted a large number  
 319 of topographic surveys on 340 watersheds in the northeastern United States with the  
 320 assistance of the Engineering Projects Administration of the Federal Bureau of  
 321 Engineering (Langbein, 1947). Benson and Horton analyzed the gullies density on  
 322 this basis and believed that the runoff moves and converges on the slope (Benson,  
 323 1959; Horton, 1936). When the erosivity of the water flow exceeds the erosion  
 324 resistance of the soil surface, the gullies would be generated and continue to develop.

325 The distance from the dividing line to the point where the erosion force equals the  
 326 erosion resistance is called the "critical distance", and the surface zone within the  
 327 critical distance is called the "non-erosion zone". One of the most important factors in  
 328 determining the width of a non-eroded zone is the infiltration capacity of the soil.  
 329 Specifically, the greater the infiltration capacity, the smaller the surface runoff. As the  
 330 infiltration capacity increases, the critical distance also increases since a larger slope  
 331 segments length is required to accumulate surface water flow of sufficient depth and  
 332 speed to begin erosion. When the infiltration capacity decreases, the surface water  
 333 flow will gradually increase correspondingly. The increase in the density of gullies  
 334 will provide a more efficient way to transport water from the surface. The formula 24  
 335 and 25 quantified this relationship that the permeability is inversely proportional to  
 336 the square of the gullies density, and this relationship is more obvious as it is closer to  
 337 the river (Jacob, 1944; Gardiner, 2010).

338 Considering the saturation-excess runoff mechanism, this study does not focus on  
 339 the process of soil infiltration capacity change with soil moisture. The saturated  
 340 hydraulic conductivity of soil, that is, the infiltration capacity when soil is saturated, is  
 341 used to quantify the rate of rainfall infiltration. The distance of each cell from gullies  
 342 can be extracted from Digital Elevation Model. Based on the work mentioned above,  
 343 the spatial distribution of gullies density could be estimated to support numerical  
 344 simulation of flow movement.

$$h = \sqrt{h_0^2 + 2 \frac{w}{K_s} \left( L_0 x - \frac{x^2}{2} \right)} \quad (24)$$

$$D^2 = \frac{w}{8 h K_s} \quad (25)$$

345 where h is the elevation of the water table at any point distance from the draining  
 346 stream (x). h<sub>0</sub> is the elevation of the draining stream, L<sub>0</sub> is the distance from the  
 347 stream to the ground-water divide. w is rate of accretion to the water table.

348

## 349 **4 MODEL APPLICATION CASE**

### 350 **4.1 Study area and data**

351 The proposed GXAJ model was tested for Tunxi watershed of 2670 km<sup>2</sup> drainage  
352 area with 11 rain gauges is located in a mountainous region with elevation ranging  
353 from 122m above sea level at the outlet to 1619m in Anhui province, China. The  
354 longest river in the watershed flows eastward to reach the outlet of watershed where  
355 the hydrological station named Tunxi is located. The long-term average annual  
356 rainfall, pan evaporation and runoff from 2007 to 2018 are 2119 mm, 770mm and  
357 1349mm, respectively. Due to the dominance of monsoon climate, more than 60% of  
358 annual rainfall occurs during May to September (flood season). The vegetation mainly  
359 consists of evergreen coniferous forests, deciduous broad-leaved forests, mixed  
360 forests, woodlands, woodland grasslands and pasture. The rainfall and discharge  
361 records of 27 flood events from the data collection network of Tunxi watershed were  
362 used to evaluate the GXAJ model's performance incorporating spatial parameters  
363 estimation mechanism (GeoPara). The spatial distribution of rainfall was obtained  
364 from interpolating the rainfall data from the 11 rain gauges (Figure 4a) using the  
365 inverse distance squared procedure.

366 With the development of remote sensing analysis and geophysical observation  
367 technical at watershed scale, a new version of Global Digital Soil Mapping System  
368 (SoilGrids250m<sup>TM</sup> V2.0, abbreviated as SoilGrids), which reflects the spatial  
369 characteristics of the subsurface, such as soil texture, has been online and applied in  
370 summer 2020 (Batjes, Ribeiro, & Oostrum, 2020). SoilGrids provides global  
371 predictions for standard numeric soil properties (organic carbon, bulk density, Cation  
372 Exchange Capacity (CEC), pH, soil texture fractions and coarse fragments) at seven  
373 standard depths (0, 5, 15, 30, 60, 100 and 200 cm) at 250m resolution, in addition to  
374 predictions of soil depth based on ca. 230,000 soil profiles data (WoSIS) and  
375 environmental layers such as climate, land cover, and topography, etc. Compared with  
376 the original version, SoilGrids V2.0 further improves the credibility and quantity of  
377 soil profile data, and could basically be used as a reliable source of soil data in  
378 mountainous areas where topographic measurements are lacking. Another data that  
379 can be easily obtained in mountainous areas is the Digital Elevation Model (DEM)

380 with 90 m spatial resolution measured jointly by NASA and NIMA (Sahoo, & Jain,  
381 2018), which is utilized in this study to depict the topography of the watershed  
382 (Figure 4).

383

#### 384 **4.2 Spatial river width estimation**

385 The spatial distribution of the river width has been extracted according to the  
386 method in 3.4 is as follows:

387 From Figure 5, several features of river width variation are obvious. The  
388 upstream region of the rivers has steep slopes where rivers are usually narrow with  
389 steep banks. Close to the watershed outlet, the gentle slope intermountain zone along  
390 with the river makes the branch less constrained by the terrain, the flattened terrain  
391 and the intersection of rivers widen the downstream channel. This is the case that  
392 wider river reaches are often located at flatter terrain with larger upstream gullies  
393 area. The predominant feature is that the river widths tend to increase with upstream  
394 gullies area while fluctuate along the channels down to the outlet.

395

#### 396 **4.3 Spatial parameters estimation**

397 According to the prior parameter estimation method proposed in this study, the  
398 spatial distribution of parameters such as SM, WM, KI, KG, CI, CG, and D have been  
399 obtained (Figure 6).

400

### 401 **5 RESULT AND DISCUSSION**

#### 402 **5.1 Flood simulation results**

403 The 27 flood events of Tunxi watershed during 2008 to 2017 were used to  
404 evaluate the performance of GXAJ model with spatial parameters estimation  
405 mechanism (GeoPara). Four indexes of relative runoff volume error (RRE, %),  
406 relative peak discharge error (RPE, %), peak time error (PTE, h) and Nash-Sutcliffe  
407 coefficient (NSC) were utilized to analyze the simulation results of GXAJ model,  
408 which were compared with that of XAJ model (Figure 7).

$$RRE = \frac{R_i - R_{obs}}{R_{obs}} \times 100\% \quad (26)$$

$$RPE = \frac{Q_{simp} - Q_{obsp}}{Q_{obsp}} \times 100\% \quad (27)$$

$$PTE = T_{simp} - T_{obsp} \quad (28)$$

$$NSC = 1 - \frac{\sum_{t=0}^{t=n} (Q_c^t - Q_{obs}^t)^2}{\sum_{t=1}^{t=n} (Q_{obs}^t - \overline{Q_{obs}})^2} \quad (29)$$

409 where  $Q_{sim}^t$  and  $Q_{obs}^t$  are simulated and measured discharge at time  $t$ , respectively.  
 410  $Q_{simp}$  and  $Q_{obsp}$  are simulated and measured flood peaks, respectively.  $T_{simp}$  and  $T_{obsp}$  are  
 411 simulated and measured flood peak time, respectively.

412 From the simulation results, the relative runoff volume error and flood peak error  
 413 of the GXAJ model are 8.4% and 10.7%, and the relative runoff volume error and  
 414 flood peak error of the XAJ model are 8.9% and 12.1%. The NSC and PTE of the  
 415 GXAJ model and the XAJ model are 0.85 and 0.88, 2.1h and 1.6h, respectively.

416 For further refined analysis of simulation results, the flood has been divided into  
 417 five phases, namely initial, rise, peak, fall and tail phase (Figure 8a). Taking the  
 418 No.2013042810 flood as an example, the flood process is considered as a function of  
 419 time ( $q=f(T)$ ), and then the first order derivative of the function is calculated  
 420 ( $Q'=\partial f(T)/\partial T$ ). The appropriate period  $\delta$  ( $\delta = 3h$ ) is adopted to smooth the derivative  
 421 process to obtain the mean linear  $Q'_{ave}$  reflecting the changing trend of  $Q'$ . As can be  
 422 seen in Figure 8a, the mean linear  $Q'_{ave}$  shows significant increasing from point A,  
 423 achieving the highest value at point B. After then,  $Q'_{ave}$  sharply decreased to the  
 424 lowest point C. Finally, the line gradually returns to its original position at point D.  
 425 Therefore, the flooding can be divided according to these points described above.

426 According to Figure 8, the GXAJ model, which considers the influence of gullies,  
 427 can reasonably simulate the characteristics of flood fluctuations, especially during the  
 428 rise phase. The flood simulated by the GXAJ model starts to rise at point A on the

429 horizontal axis, leaping from 53 m<sup>3</sup>/s to 94 m<sup>3</sup>/s, which is basically close to the  
430 measured change from 54 m<sup>3</sup>/s to 101 m<sup>3</sup>/s. In the rise phase, although the simulated  
431 results are somewhat larger compared to the measured data, with RRE of about 24%,  
432 the rising trend is reasonably simulated, which has been illustrated by NSC of 0.71.  
433 The RRE is significantly reduced to 7.5% in the critical stage of the flood simulation,  
434 that is, the peak phase. Meanwhile, an appropriate process simulation was achieved,  
435 resulting in NSC of 0.98. In the first half of the fall phase, the simulation results  
436 remain fine, but in the second half of the fall phase, noticeable simulation errors start  
437 to appear, bringing the RRE to 35%. During the rise phase, as the intensity of rainfall  
438 increases, gullies on the slope can provide an efficient pathway of water conveyance,  
439 enabling a rapid rise in discharge in the river and watershed outlet. At the beginning  
440 of the flood fall, although the rainfall has stopped, there is still water flow in the river,  
441 which is conducive to the rapid transportation of the remaining water on the slope.  
442 The reduction in flow would result in a gradual decline in velocity, making the tail  
443 end of the flood recede at a lower rate.

444

## 445 **5.2 Rationality analysis of parameters**

446 The rationality of spatial distribution of parameters is one of the critical issues to  
447 ensure the dynamic simulation of hydrological factors such as soil moisture and water  
448 flow by the model (Tong, Li, Yao, & Huang, 2018; Kim, Lee, Kim, & Choi, 2016).  
449 The correlation between parameters including free storage capacity (SM) and gullies  
450 density (D) with factors such as elevation and distance from the river was further  
451 explored. Given that the undulating topography is concentrated in the southwestern  
452 part of the watershed, a focus area of alternating valleys and ridges was set there,  
453 which is indicated by the red boxes in Figure 9 and Figure 11 . The quantitative  
454 relationships between parameters such as SM and D and geographic elements  
455 including elevation (DEM) and distance from river (Dis) in the focus area are shown  
456 in Figure 10 and Figure 12.

457 Figure 9 and Figure 10 show that the value of SM first decreases and then

458 increases with longer distance from the river and higher elevation. In the bank area  
459 located at the bottom of the valley, the soil layer containing sediment is thick, and has  
460 a high water storage capacity. In the slope section far from the river, erosion caused a  
461 thin soil layer with a small organic matter content and SM value. However, along the  
462 mountain ridge with high altitude, the value of SM increases with the thicker soil and  
463 larger organic matter content. Therefore, in the area of focus, the SM values of bank  
464 and mountain ridges are larger than that of the middle of the slope segments.

465 As can be seen from Figure 11 and Figure 12, the gullies density D values near  
466 the ridges are smaller than that of the areas along the river. The rainfall can enter into  
467 soil easily due to strong infiltration capacity of thick leaf litter and humus layer along  
468 mountain ridges. Specifically, the greater the infiltration capacity, the less surface  
469 runoff contributes to soil erosion. Thus, a longer critical distance is required to  
470 accumulate the flow needed to form the gullies, which implies lower gullies density.  
471 Close to the river, the proportion of fine sediment gradually increases and the  
472 infiltration capacity decreases, contributing to the generation of surface runoff.  
473 Moreover, the runoff carrying sediment from the upper slopes is discharged into the  
474 river through the bank area. These factors mentioned above promote the phenomenon  
475 that gullies density values increase with shorter distances from the river, which is  
476 consistent with the views on the spatial characteristics of gullies proposed by Horton  
477 and Benson et al. (Benson, 1959; Horton, 1936; Raphaël, Paolo, Giulia, Parlange, &  
478 Andrea, 2016; Godsey, & James, 2015).

479

### 480 **5.3 Parameters sensitivity**

481 The sensitivities of SM and D in various flood phases are analyzed, which could  
482 inform the need for dynamic adjustment of parameters in further potential real-time  
483 forecasting and facilitate fine simulation of rainfall-runoff processes. First, the main  
484 parameters of the GXAJ model for the Tunxi watershed were prior estimated based on  
485 data on soil texture and topography. Then, the parameters SM and D varied in the  
486 range of 0-30 and 0.1-15 in step of 0.1, respectively, to participate in the simulation of

487 No.2013042810 flood. The results are statistically evaluated by RRE, RPE, PTE and  
488 NSC. Furthermore, the NSC and RRE are utilized to quantify the influence of  
489 parameters SM and D on the rise, peak, fall and tail phases, respectively (Figure 13  
490 and Figure 14).

491 According to Figure 15 , in various phases of the flood process, for SM and D,  
492 the differences may lead to the largest variations in RRE in the rise phase, followed  
493 by fall, peak and tail phase, successively. The sensitivity of SM is obvious in the rise  
494 phase, but not in the rest of the phases. Parameter D has significant effect on the rise  
495 phase of the flood, and the effect on the fall phase can also be identified. Both  
496 parameters SM and D can have a large impact on the RRE during the flooding phase,  
497 where the RRE is more likely to vary with the change of D. From Figure 15 , D have  
498 strong impact on NSC in the rise phase and a slight effect in the flood peak phase. On  
499 the contrary, the parameter SM has a significantly influence on the flood peak, but  
500 tiny influence on the flood rise, fall and tail. It can be concluded that D has the  
501 noticeable influence on the RRE and NSC during the rise phase, while SM has a  
502 significant effect on the RRE in the flood rise phase and NSC during peak phase,  
503 respectively.

504

#### 505 **5.4 Dynamic change of soil moisture and channel flow**

506 In addition to the hydrograph in the river, the GXAJ model can be used to  
507 reasonably simulate the spatial dynamics of free water content and channel flow at the  
508 watershed scale, which is one of capabilities beyond XAJ model. Taking the  
509 No.2013042810 flood as an example, the soil free water content simulated by GXAJ  
510 model have been illustrated at the time of 35h, 45h, 55h, 65h ,75h and 85h (Figure  
511 16). It can be seen that the free water content of soil is low before the occurrence of  
512 rainfall. Following the rainfall, free water content increases gradually and reaches  
513 saturation state before the flood peak appears. After the rainfall stop, the free water  
514 content decreases and finally stabilizes in a certain point which is slightly higher than  
515 that at the rainfall occurrence.

516 Compared to soil water content, flow in gullies change more rapidly and are  
517 hardly graphed. The continuous heavy rainfall would cause the streams in the gullies  
518 to resemble the flow in the primary river, instead of the original trickle on the slope.  
519 In the rainfall-runoff process, the distance between the cell where this phenomenon  
520 occurs and the river changes dynamically, so that the flow in the gullies keeps  
521 extending towards the uphill during the rise of the flood and dissipates towards the  
522 downhill when the flood falls. For graphing dynamic change of gullies flow  
523 appropriately, the flow index  $\mu$  was utilized to tell the cell in which gullies flow is  
524 strong or not. Specifically, the cell where the gully is located is highlighted (Figure  
525 17) when the gullies flow exceeds  $\mu$ , the value of which is influenced by climate and  
526 topography. In the case of sufficient rainfall and steeper slopes in the watershed, the  
527 water flows are more likely to converge to promote the formation of channels.  
528 Therefore, based on the hourly rainfall data of Tunxi watershed in the flood season  
529 from 2008 to 2017, the runoff generated in each cell have been calculated to  
530 determine flow index  $\mu$  with the consideration of terrain slope.

$$\mu = \frac{1}{K} \sum_{k=1}^K \sum_{b=1}^{NS} (R_{k,b} \times S_{k,b}) = \sum_{g=1}^{GS} (R_g \times S_g) \quad (30)$$

531 where NS is the quantity of cells converging to the source points of river system, b is  
532 the index of cells converging to the source points, from 1 to NS; K is the quantity of  
533 source points in the river system, and k is the index of source points, from 1 to K.  $S_{k,b}$   
534 and  $R_{k,b}$  are respectively the slope and runoff of the cell numbered b. GS is the  
535 quantity of cells converging to the highlighted cell during rainfall-runoff process.

536

## 537 **6 CONCLUSION**

538 The gullies system composed of shallow furrows, trenches and well-developed  
539 grooves has been systematically generalized in Improved Distributed Grid-Xinanjiaog  
540 model (GXAJ). Within the watershed divided into a series of orthogonal cells, the  
541 shallow furrows and trenches inside cells are conceptualized as primary gullies, and

542 the well-developed grooves between cells are considered as the main gullies. The  
543 runoff generated in the cell would first enters the primary gullies using the kinetic  
544 wave equation considering the gullies density, and then flows through multiple cells  
545 in the main gullies to enters river. Based on the soil data provided by the Global  
546 Digital Soil Mapping System (SoilGrids), a parameter estimation scheme (GeoPara)  
547 has been proposed to support the simulation of 27 flood events in the Tunxi watershed  
548 of Anhui Province by GXAJ model, of which the simulation results were compared  
549 with that of XAJ model and measurement. According to the statistical analysis, the  
550 error level of peak error and Nash-Sutcliffe coefficient (NSC) of GXAJ model and  
551 XAJ model are 10.7% and 12.1%, 0.85 and 0.88, respectively. For further refined  
552 analysis of simulation results, the flood has been divided into five phases, namely  
553 initial, rise, peak, fall and tail phases. Specifically, although the GXAJ model  
554 overestimates the discharge by 24% in the flood rise phase, the rising trend is  
555 reasonably simulated with the NSC of 0.71. Meanwhile, the simulation results of  
556 GXAJ model considering the influence of gullies are basically consistent with the  
557 measurement in flood peak phase, illustrated by RRE of 7.5% and NSC of 0.98. It can  
558 be considered that The GXAJ model enables a reasonable simulation of floods,  
559 especially the flood peak.

560 Sensitivity analysis of the free water storage capacity SM and gullies density D  
561 are conducted in various flood phases. For SM and D, the differences may lead to the  
562 largest variations in RRE in the rise phase, followed by fall, peak and tail phase,  
563 successively. It can be concluded that D has the noticeable influence on the RRE and  
564 NSC during the rise phase, while SM has a significant effect on the RRE in the flood  
565 rise phase and NSC during peak phase, respectively. In addition to the hydrograph,  
566 spatial dynamics of free water content and channel flow at the watershed scale could  
567 be simulated reasonably by GXAJ model. It can be seen that the free water content of  
568 soil is low before the occurrence of rainfall. Following the rainfall, free water content  
569 increases gradually and reaches saturation state before the flood peak appears. After  
570 the rainfall stop, the free water content decreases and finally stabilizes in a certain

571 point which is slightly higher than that at the rainfall occurrence. Compared with the  
572 soil water content, the flow in gullies changes rapidly, extending towards the uphill  
573 during the rise of the flood and dissipates towards the downhill when the flood falls.

574

#### 575 **DATA AVAILABILITY STATEMENT**

576 The data collected from the gauging stations including rainfall and streamflow are  
577 available on request from the corresponding author, which are not publicly available  
578 due to privacy or ethical restrictions. The soil data from Global Digital Soil Mapping  
579 System are openly available at <https://doi.org/10.1371/journal.pone.0169748>  
580 (Tomislav et al, 2017). The high-resolution terrain data used in this study are openly  
581 available at <https://doi.org/10.1016/j.cageo.2017.10.001> (Sahoo, R., & Jain, V. 2018).

582

#### 583 **REFERENCES**

584 Abbaszadeh, P., Gavahi, K., & Moradkhani, H. (2020). Multivariate remotely  
585 seNSCd and in-situ data assimilation for enhancing community WRF-Hydro model  
586 forecasting. *Advances in Water Resources*, 145, 103721.

587 <https://doi.org/10.1016/j.advwatres.2020.103721>.

588 Ajami, Newsha, K., Duan, Q., Gao, X., Sorooshian, & Soroosh. (2006). Multimodel  
589 combination techniques for analysis of hydrological simulations: application to  
590 distributed model intercomparison project results. *Journal of Hydrometeorology*, 7,  
591 755-768.

592 <https://doi.org/10.2307/2060650>.

593 Bao, H.J., Zhao, L.N., He, Y., Li, Z.J., Wetterhall, F., Cloke, H. L. .... Manful, D.  
594 (2011). Coupling eNSCmble weather predictions based on TIGGE database with  
595 Grid-Xinjiang model for flood forecast. *Advances in Geosciences*, 29(30), 61-67.

596 <https://doi.org/10.5194/adgeo-29-61-2011>.

597 Batjes, N. H., Ribeiro, E., & Oostrum, A. V. (2020). Standardised soil profile data to  
598 support global mapping and modelling (WoSIS snapshot 2019). *Earth System Science*  
599 *Data*, 12, 299-320.

600 <https://doi.org/10.5194/essd-12-299-2020>.

601 Benson, M. A. (1959). Channel-Slope Factor in Flood-Frequency Analysis. *Journal of*  
602 *the Hydraulics Division*, 85(1), 1-9.

603 Daniel, B. W., Guo, Y., & John, F. E. (2020). Six decades of rainfall and flood  
604 frequency analysis using stochastic storm transposition: Review, progress, and  
605 prospects. *Journal of Hydrology*, 585, 124816.  
606 <https://doi.org/10.1016/j.jhydrol.2020.124816>.

607 Diaz, R. G. Analysis of Manning coefficient for small-depth flows on vegetated beds.  
608 (2005). *Hydrological Processes*, 19(16), 3221-3233.  
609 <https://doi.org/10.1002/hyp.5820>.

610 Fraga, I., Cea, L., & Puertas, J. (2019). Effect of rainfall uncertainty on the  
611 performance of physically based rainfall-runoff models. *Hydrological Processes*, 33,  
612 160-173. <https://doi.org/10.1002/hyp.13319>.

613 Garbrecht, J., & Martz, L. W. (1997). The assignment of gullies direction over flat  
614 surfaces in raster Digital Elevation Models. *Journal of Hydrology*, 193(4), 204-213.  
615 [https://doi.org/10.1016/S0022-1694\(96\)03138-1](https://doi.org/10.1016/S0022-1694(96)03138-1).

616 Gardiner, V. (2010). Estimation of gullies density from topological variables. *Water*  
617 *Resources Research*, 15(4), 909-917.  
618 <https://doi.org/10.1029/wr015i004p00909>

619 Godsey, S. E., & James, W. K. (2015). Dynamic, discontinuous stream networks:  
620 hydrologically driven variations in active gullies density, flowing channels and stream  
621 order. *Hydrological Processes*, 28(23), 5791-5803.  
622 <https://doi.org/10.1002/hyp.10310>.

623 Grunwald, S., Thompson, J. A., & Boettinger, J. L. (2011). Digital soil mapping and  
624 modeling at continental scales: Finding solutions for global issues. *Soil Science*  
625 *Society of America Journal*, 75, 1201-1213.  
626 <https://doi.org/10.2136/sssaj2011.0025>.

627 Gupta, H. V., Sorooshian, S., & Yapo, P. O. (1999). Status of automatic calibration for  
628 hydrologic models: comparison with multilevel expert calibration, *Journal of*  
629 *Hydrologic Engineering*, 4(2), 135-143.  
630 [https://doi.org/10.1061/\(ASCE\)1084-0699\(1999\)4:2\(135\)](https://doi.org/10.1061/(ASCE)1084-0699(1999)4:2(135)).

631 Gupta, H.V., Wagener, T., & Liu, Y. (2008), Reconciling theory with observations:  
632 elements of a diagnostic approach to model evaluation. *Hydrological Processes*,  
633 22(18), 3802-3813.  
634 <https://doi.org/10.1002/hyp.6989>

635 Haney, R. L., SeNSCman, S. A., Hons, F. M., & Zuberer, D. A. (2000). Effect of  
636 glyphosate on soil microbial activity and biomass. *Weed Science*, 48(1):89-93. [https://doi.org/10.1614/0043-1745\(2000\)048\[0089:EOGOSM\]2.0.CO;2](https://doi.org/10.1614/0043-1745(2000)048[0089:EOGOSM]2.0.CO;2).

638 HaNSCn, C., Shiva, J. S., Mcdonald, S., & Nabors, A. (2019). Assessing retrospective  
639 National Water Model streamflow with respect to droughts and low flows in the  
640 Colorado River basin. *Journal of the American Water Resources Association*, 55(4).  
641 <https://doi.org/10.1111/1752-1688.12784>.

642 Horritt, M. S., & Bates, P. D. (2001). Predicting floodplain inundation: raster-based  
643 modelling versus the finite-element approach. *Hydrological Processes*, 15, 825-842.  
644 <https://doi.org/10.1002/hyp.188>.

645 Horton, Robert, E. (1936). Maximum groundwater levels. *Eos Transactions American*  
646 *Geophysical Union*, 17(2), 344-357.  
647 <https://doi.org/10.1029/TR017i002p00344>.

648 Jacob, C. E. (1944). Correlation of ground-water levels and precipitation on Long  
649 Island, New York. 25(6), 928-939. <https://doi.org/10.1029/TR025i006p00928>.

650 Kim, D., Lee, J., Kim, H. & Choi, M. (2016). Spatial composition of AMSR2 soil  
651 moisture products by conditional merging technique with ground soil moisture data.  
652 *Stochastic Environmental Research & Risk Assessment*, 30, 2109-2126.  
653 <https://doi.org/10.1007/s00477-016-1300-0>.

654 Kim, S. B., Jakob, J., Johnson, T., Moghaddam, M., Tsang, L., Colliander, A., ... Yueh,  
655 S. H. (2017). Surface soil moisture retrieval using the L-band synthetic aperture radar  
656 onboard the soil moisture active-passive satellite and evaluation at core validation  
657 sites. *IEEE Transactions on Geoscience and Remote Sensing*, 55(4), 1897-1914.  
658 <https://doi.org/10.1109/TGRS.2016.2631126>.

659 Kirkby, M. J. (1978). *Hillslope hydrology*. Wiley.  
660 <https://doi.org/10.1002/esp.3290070112>.

661 Koren, V., Reed, S., & Smith M. (2004). Hydrology laboratory research modeling  
662 system (HL-RMS) of the US national weather service. *Journal of Hydrology*, 291,  
663 297-318.  
664 <https://doi.org/10.1016/j.jhydrol.2003.12.039>.

665 Kuo, W. L., Steenhuis, T. S., Mcculloch, C. E., Mohler, C. L., Weinstein, D. A.,  
666 Degloria, S. D., & Swaney, D. P. (1999). Effect of grid size on runoff and soil  
667 moisture for a variable-source-area hydrology model. *Water Resources Research*,  
668 35(11), 3419–3428.  
669 <https://doi.org/10.1029/1999WR900183>.

670 Lahmers, T. M., Gupta, H., Castro, C. L., Gochis, D. J., & Hazenberg, P. (2019).  
671 Enhancing the structure of the WRF-Hydro hydrologic model for semi-arid  
672 environments. *Journal of Hydrometeorology*, 20(4), 691–714.  
673 <https://doi.org/10.1175/JHM-D-18-0064.1>.

674 Lai, C., Chen, X., Wang, Z. L., Yu H.J. & Bai, X. Y. (2020). Flood risk assessment  
675 and regionalization from past and future perspectives at basin scale. *Risk Analysis*,  
676 40(7), 1399-1417.  
677 <https://doi.org/10.1111/risa.13493>.

678 Langbein, W. B. (1947). Topographic characteristics of gullies basins: U.S. Geol.  
679 Survey Water-Supply Paper. 968(C), 125-157.

680 Levia, D. F., & Frost, E. E. (2003) A review and evaluation of stemflow literature in  
681 the hydrologic and biogeochemical cycles of forested and agricultural ecosystems.  
682 *Journal of Hydrology*, 274, 1-29.  
683 [https://doi.org/10.1016/S0022-1694\(02\)00399-2](https://doi.org/10.1016/S0022-1694(02)00399-2).

684 Li, Z. J., Liang, K., Kan, G. Y., Li Q. L., Yao, C., & Zhang, K. (2016). A method for  
685 deriving the river network flow concentration parameter Cs of the Xin'anjiang model.  
686 *Advances in Water Science*, 27(5), 652-661.  
687 <https://doi.org/10.14042/j.cnki.32.1309.2016.05.002>.

688 Li, Z. J., Yao, C., Zhang, K., Zhu, Y. L., Liu Zhiyu, Li Qiaoling., ..., Huang, P. N.  
689 (2017). Research and application of the high-resolution rainfall runoff hydrological  
690 model in flood forecasting. *Journal of Hohai University (Natural Sciences)*, 45(6),

691 471-479.  
692 <https://doi.org/10.3876/j.issn.10001980.2017.06.001>.  
693 Liang, C., Li, D. Q., Yuan, Z. J., & Liao, Y., S. (2019). Assessing urban flood and  
694 drought risks under climate change, China. *Hydrological Processes*, 33(9), 1349-  
695 1361. <https://doi.org/10.1002/hyp.13405>.  
696 Liang, D., Lu, J. Z., Chen, X.L., Liu C., & Lin, J.L. (2020). An investigation of the  
697 hydrological influence on the distribution and transition of wetland cover in a  
698 complex lake–floodplain system using time-series remote sensing and hydrodynamic  
699 simulation. *Journal of Hydrology*, 587, 125038.  
700 <https://doi.org/10.1016/j.jhydrol.2020.125038>.  
701 Martz, L. W., & Garbrecht, J. (1995). Automated recognition of valley lines and  
702 gullies networks from grid Digital Elevation Models: a review and a new method-  
703 comment. *Journal of Hydrology*, 139(4), 263-293.  
704 [https://doi.org/10.1016/0022-1694\(94\)02619-M](https://doi.org/10.1016/0022-1694(94)02619-M).  
705 Mcclean, F., Dawson, R., & Kilsby, C. (2020). Implications of using global Digital  
706 Elevation Models for flood risk analysis in cities. *Water Resources Research*, 56(10),  
707 e2020WR028241.  
708 <https://doi.org/10.1029/2020WR028241>.  
709 Mohammad, A., & Seyed, M. H. (2019). A simple innovative method for calibration  
710 of Manning’s roughness coefficient in rivers using a similarity concept. *Journal of*  
711 *Hydrology*, 575, 810-823.  
712 <https://doi.org/10.1016/j.jhydrol.2019.05.083>.  
713 Montgomery, D. R., & Dietrich, W. E. (2010). Source areas, gullies density, and  
714 channel initiation. *Water Resources Research*, 25(8), 1907-1918.  
715 <https://doi.org/10.1029/WR025i008p01907>  
716 Najafi, M. R., Moradkhani, H., & Jung, I. W. (2011). Assessing the uncertainties of  
717 hydrologic model selection in climate change impact studies. *Hydrological Processes*,  
718 25(18), 2814-2826.  
719 <https://doi.org/10.1002/hyp.8043>.  
720 Raphaël, M., Paolo, T., Giulia, S., Parlange, M. B., & Andrea, R. (2016). Field study

721 on gullies densities and rescaled width functions in a high-altitude alpine catchment.  
722 *Hydrological Processes*, 30(13), 2138-2152.  
723 <https://doi.org/10.1002/hyp.10783>.

724 Reed, S., Koren, V., Smith M., Zhang, Z., Moreda, F., Seo, D. J., & Participants, A. D.  
725 (2004) Overall distributed model intercomparison project results. *Journal of*  
726 *Hydrology*, 298, 27-60.  
727 <https://doi.org/10.1016/j.jhydrol.2004.03.031>.

728 Sahoo, R., & Jain, V. (2018). Sensitivity of gullies morphometry based hydrological  
729 respoNSC (GIUH) of a river basin to the spatial resolution of DEM data -  
730 ScienceDirect. *Computers & Geosciences*, 111, 78-86.  
731 <https://doi.org/10.1016/j.cageo.2017.10.001>.

732 Saxton, K. E., & Rawls, W. J. (2006). Soil water characteristic estimates by texture  
733 and organic matter for hydrologic solutions. *Soil Physics*, 70, 1569-1578.  
734 <https://doi.org/10.2136/sssaj2005.0117>.

735 Sorooshian, S., Duan, Q., & Gupta, V. K. (1993). Calibration of rainfall-runoff  
736 models: Application of global optimization to the Sacramento Soil Moisture  
737 Accounting Model. *Water Resources Research*, 29(4), 1185-1194.  
738 <https://doi.org/10.1029/92wr02617>.

739 Sun, T. T., Wang, Y. G., Hui, D. F., Jing, X., & Feng, W. T. (2020). Vertical  
740 distributions of soil microbial biomass carbon: a global dataset. *Data in Brief*, 32:  
741 106147.  
742 <https://doi.org/10.1016/j.dib.2020.106147>.

743 Thielen, A. H., Lücke, A., Diekkrüger, B., & Richter, O. (1999). Scaling input data by  
744 GIS for hydrological modelling. *Hydrological Processes*, 13(4), 611-630.  
745 [https://doi.org/10.1002/\(SICI\)1099-1085\(199903\)13:4<611::AID-HYP758>3.0.CO;2-](https://doi.org/10.1002/(SICI)1099-1085(199903)13:4<611::AID-HYP758>3.0.CO;2-6/)  
746 [6/](https://doi.org/10.1002/(SICI)1099-1085(199903)13:4<611::AID-HYP758>3.0.CO;2-6/).

747 Tomislav, H., Jorge, M. D. J., Heuvelink, G. B. M., Maria, R.G., Milan, K.,  
748 Aleksandar, B., ...Bernhard, B. M. (2017). SoilGrids250m: Global gridded soil  
749 information based on machine learning. *PLoS One*, 12(2), e0169748.  
750 <https://doi.org/10.1371/journal.pone.0169748>.

751 Tong, B. X., Li, Z. J., Wang, J. F., Yao, C., & He, M. (2020). Development of  
752 topography-based river width estimation model for medium-sized mountainous  
753 watersheds. *Journal of hydrologic engineering*, 25, 1-11.  
754 [https://doi.org/10.1061/\(ASCE\)HE.1943-5584.0001888](https://doi.org/10.1061/(ASCE)HE.1943-5584.0001888)

755 Tong, B. X., Li, Z. J., Yao, C., & Huang, Y. C. (2018). Derivation of the spatial  
756 distribution of free water storage capacity based on topographic index. *Water*, 10  
757 (10):1-15.  
758 <https://doi.org/10.3390/w10101407>.

759 Viterbo, F., Mahoney, K., Read, L., Salas, F., & Cifelli, R. (2020). A multiscale,  
760 hydrometeorological forecast evaluation of National Water Model. *Journal of*  
761 *Hydrometeorology*, 21, 475-499.  
762 <https://doi.org/10.1175/JHM-D-19-0125.1>.

763 Wang, C., Fu, B., Zhang, L., & Xu Z. H. (2019). Soil moisture-plant interactions: an  
764 Ecohydrological review. *Journal of Soils & Sediments*, 19, 1-9.  
765 <https://doi.org/10.1007/s11368-018-2167-0>.

766 Wang, D. F., Xu, H. D., Wang, X. L., & Wu, X. (2020). Statistical analyses of the  
767 effect of a gullies tunnel on landslide hydrogeological characteristics. *Hydrological*  
768 *Processes*, 34(1), 2418-2432.  
769 <https://doi.org/10.1002/hyp.13738>.

770 Wang, P. L., & Zhao, R. J. (1989). Optimazation method of calibration of Xianjing  
771 Model (3 Components). *Journal of Hohai University (Natural Science Edition)*, 1989,  
772 04: 65-69.  
773 <https://doi.org/CNKI:SUN:HHDX.0.1989-04-009>.

774 Xia, J., Wang, H. Y., Gan, Y. Y., et al. (2019). Research progress in forecasting  
775 methods of rainstorm and flood disaster in China. *Torrential Rain and Disasters*,  
776 38(5), 416-421. <https://doi.org/10.3969/j.issn.1004-9045>.

777 Xiang, T., Vivoni, E. R., Gochis, D. J., & Mascaro, G. (2017). On the diurnal cycle of  
778 surface energy fluxes in the North American monsoon region using the WRF-Hydro  
779 modeling system. *Journal of Geophysical Research: Atmospheres*, 122, 9024-9049.  
780 <https://doi.org/10.1002/2017JD026472>.

781 Yao, C., Li, Z. J., Yu, Z. Z., & Zhang, K. (2012). A priori parameter estimates for a  
782 distributed, grid-based xinanjiang model using geographically based information.  
783 *Journal of Hydrology*, 468(25), 47-62.  
784 <https://doi.org/10.1016/j.jhydrol.2012.08.025>.

785 Yao, C., Li, Z. J.; Bao, H. J.; & Yu, Z. B. (2009). Application of a Developed Grid-  
786 Xinanjiang Model to Chinese Watersheds for Flood Forecasting Purpose. *Journal of*  
787 *Hydrologic Engineering*, 14(9), 923-934.  
788 [https://doi.org/10.1061/\(ASCE\)HE.1943-5584.0000067](https://doi.org/10.1061/(ASCE)HE.1943-5584.0000067)

789 Zeng, Z. Y., Xu, J. J., & Wan, Y. Q. (2020). Advances in flood risk identification and  
790 dynamic modelling based on remote sensing spatial information. *Advances in Water*  
791 *Science*, 31(3), 463-472.  
792 <https://doi.org/10.14042/j.cnki.32.1309.2020.03.016>

793 Zhang, G.T., Cui, P., Yin Y. Z., Wen, J., Wang, H., Yan, Y., ... Wang, J. (2019). Real-  
794 time monitoring and estimation of the discharge of flash floods in a steep mountain  
795 catchment. *Hydrological Processes*, 33(25), 3195-3212.  
796 <https://doi.org/10.1002/hyp.13551>.

797 Zhang, J. Q., Lin, P. R., Gao, S. & Fang, Z. (2020). Understanding the re-infiltration  
798 process to simulating streamflow in north central Texas using the WRF-Hydro  
799 modeling system. *Journal of Hydrology*, 587, 124902.  
800 <https://doi.org/10.1016/j.jhydrol.2020.124902>.

801 Zhao, R. J. (1984). Watershed hydrological simulation: Xinanjiang model and  
802 Shanbei model. Beijing: Water Resources and Electric Power Press.

803

804 **TABLES**

805 Table 1 Main parameters of GXAJ model

| <b>parameter</b> | <b>meaning</b>                 | <b>unit</b> | <b>parameter</b> | <b>meaning</b>                        | <b>unit</b>      |
|------------------|--------------------------------|-------------|------------------|---------------------------------------|------------------|
| <i>SM</i>        | free water storage capacity    | mm          | <i>CI</i>        | regression coefficient of interflow   | -                |
| <i>WM</i>        | tension water storage capacity | mm          | <i>CG</i>        | regression coefficient of groundwater | -                |
| <i>KI</i>        | outflow coefficient of         | mm          | <i>D</i>         | gullies density                       | km <sup>-1</sup> |

| <i>KG</i> | interflow<br>outflow coefficient of<br>groundwater | mm | <i>Mn</i> | Manning roughness | - |
|-----------|--|----|-----------|-------------------|---|
|-----------|--|----|-----------|-------------------|---|

806

807 **FIGURE LEGENDS**

808 **FIGURE 1.** Schematic of water movement. The watershed has been divided into a  
809 series of orthogonal cells where the overland flow occurs during rainfall. Shallow  
810 furrows and trenches inside the cells are conceptualized as primary gullies, in which  
811 water movement is simulated by kinematic wave equation considering the gullies  
812 density, and well-developed grooves between cells are considered as main gullies  
813 where water moves as a kinematic wave and enters the river to participate in flood  
814 routing.

815 **FIGURE 2.** Flowcharts of parameter estimation. Input data, topographic  
816 characteristics and parameters are colored in green, orange and blue respectively.

817 **FIGURE 3.** Vertical distribution of organic matter has been shown schematically.  $\alpha$   
818 and  $\beta$  are ratio of the organic matter content within the possible thinnest and thickest  
819 layer to the total organic matter, respectively, which could be obtained according to  
820 the field survey and operational experience of underlying surface characteristics  
821 generalization in rainfall-runoff process simulation in the watershed.

822 **FIGURE 4.** Observation stations and geographical data of Tunxi watershed. The  
823 digital elevation and observation station network is shown in (a), from west to east,  
824 the rainfall stations are Zuo Long, Cheng cun, Da liang, Qian xian, Shang xikou, Yan  
825 qian, Ru cun, Xiu ning, Wu cheng, Tunxi and Shi men in order. The hydrological  
826 station Tunxi is located at the outlet of the watershed. (b) Thickness of the soil  
827 aeration zone. Mass fraction of Sand, Silt and Clay have been shown in (c), (d) and  
828 (e), respectively. (f) Organic matter in  $\text{dg/kg}^{-1}$ .

829 **FIGURE 5.** Spatial distribution of river widths and terrain slope within Tunxi  
830 watershed. Considering the difficulty of showing narrow rivers with lines on a small-  
831 scale cell map, the cells through which the river flows are converted into points whose  
832 size can be used to quantify the river width. To make the legend length appropriate,  
833 the size of the points are divided into five sections: 12-20, 20-40, 40-60, 65-90 and

834 90-147 meters. Areas with low slope are marked in green and those with high slope  
835 are in red.

836 **FIGURE 6.** Spatial parameters of Tunxi watershed. (a) Free water storage capacity,  
837 SM. (b) Sum of tension water capacity in upper, lower and deep layers. (c) The  
838 distribution of WUM, and the ratio of WUM to WM can be roughly estimated from  
839 the ratio of active layer to soil vadose zone. (d) Based on operational experience, the  
840 value of WDM that has little effect on hourly-scale flooding is generally about 40 to  
841 50% of WM, and thus can be distinguished from WM. (e) Outflow coefficient of  
842 interflow, KI. (f) Outflow coefficient of groundwater, KG. (g) Regression coefficient  
843 of interflow, CI. (h) Regression coefficient of groundwater, CG. (i) Gullies density, D.

844 **FIGURE 7.** Flood simulation results. The results of the GXAJ model and the XAJ  
845 model are shown on the horizontal and vertical axes, respectively, with a 45-degree  
846 angle divider for visual comparison. For error metrics such as RRE, RPE, and PTE,  
847 the points on the left side of the dividing line indicate that the GXAJ model performs  
848 better than the XAJ model. For NSC, the conclusion is reversed.

849 **FIGURE 8.** Various phases of No.2013042810 flood. (a) The flood process is colored  
850 in blue. The first order derivative of the flood process is  $Q'$ , being represented by the  
851 black line. The red line  $Q'_{ave}$  is the result of smoothing the black line and is mainly  
852 used to reflect the trend of  $Q'$ . The  $Q'_{ave}$  shows significant increasing from point A,  
853 achieving the highest value at point B. After then,  $Q'_{ave}$  sharply decreased to the  
854 lowest point C. Finally, the line gradually returns to its original position at point D.  
855 With reference to these points mentioned, the flood can be divided into initial, rise,  
856 peak, fall and tail phase phases. (b) The measured results and GXAJ model  
857 simulations for flood 2013042810 are shown by black and red lines, respectively. The  
858 inverse scale on the right vertical axis is used to quantify the precipitation, which is  
859 represented by the blue bar.

860 **FIGURE 9.** Spatial analysis of SM. (a) Digital elevation the Tunxi watershed. (b) is  
861 an enlargement of the area in the red box of (a). (c) shows the spatial distribution of  
862 free storage capacity. (d) is also a zoomed-in view of a local area, similar to (b), with

863 the purpose of presenting the distribution of SM parameters among multiple  
864 tributaries.

865 **FIGURE 10.** Rationality analysis of SM. (a) The correlation between free water  
866 storage capacity and elevation. (b) The correlation between free water storage  
867 capacity and distance from river.

868 **FIGURE 11.** Spatial analysis of D. (a) The distance from the river within the Tunxi  
869 watershed has been shown. The area in the red box is enlarged in (b). (c) is a map of  
870 the spatial distribution of gullies density, and (d) is a zoomed-in view of the area in  
871 the red box of (c). Rivers have been labeled to visualize the distance to the river in  
872 any cell.

873 **FIGURE 12.** Rationality analysis of D. (a) The correlation between gullies density  
874 and elevation. (b) The correlation between gullies density and distance from river.

875 **FIGURE 13.** Sensitivity of SM at various flood phases. The change of RRE, RPE,  
876 PTE and NSC with parameter SM of the whole process of flood have been shown in  
877 first row. The following rows have revealed the change of RRE and NSC with SM in  
878 rise, peak, fall and tail phases, respectively.

879 **FIGURE 14.** Sensitivity of D at various flood phases. RRE, RPE, PTE and NSC of  
880 the whole process of flood have been shown in first row. The following rows have  
881 revealed the change of RRE and NSC with gullies density in rise, peak, fall and tail  
882 phases, respectively.

883 **FIGURE 15.** Parameter sensitivity analysis. (a) Variations in RRE with changes in D  
884 and SM during the rise, peak, fall, and tail phases, respectively. (b) Variance of NSC  
885 with changes in D and SM during the rise, peak, fall, and tail phases, respectively.

886 **FIGURE 16.** Dynamic change of soil free water content, simulated by GXAJ model  
887 at the time of 35h, 45h, 55h, 65h, 75h and 85h.

888 **FIGURE 17.** Dynamic change of flow in channel system consisted of rivers and  
889 gullies, simulated by GXAJ model at the time of 35h, 45h, 55h, 65h, 75h and 85h

890