# Identifying discontinuities of flood frequency curves

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# Abstract

Discontinuities in flood frequency curves, here referred to as flood divides, hinder the estimation of rare floods. In this paper we develop an automated methodology for the detection of flood divides from observations and models, and apply it to a large set of case studies in the USA and Germany. We then assess the reliability of the PHysically-based Extreme Value (PHEV) distribution of river flows to identify catchments that might experience a flood divide, validating its results against observations. This tool is suitable for the identification of flood divides, with a high correct detection rate especially in the autumn and summer seasons. It instead tends to indicate the emergence of flood divides not visible in the observations in spring and winter. We examine possible reasons of this behavior, finding them in the typical streamflow dynamics of the concerned case studies. By means of a controlled experiment we also re-evaluate detection capabilities of observations and PHEV after discarding the highest maxima for all cases where both empirical and theoretical estimates display flood divides. PHEV mostly confirms its capability to detect a flood divide as observed in the original flood frequency curve, even if the shortened one does not show it. These findings prove its reliability for the identification of flood divides and set the premises for a deeper investigation of physiographic and hydroclimatic attributes controlling the emergence of discontinuities in flood frequency curves.

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# 6 Highlights:

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- We develop an automated method to detect discontinuities of flood frequency curves
- We test it on observed and physically-based theoretical flood frequency curves
- We discuss the reliability of the physically-based approach to detect discontinuities

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## 10 Abstract

Discontinuities in flood frequency curves, here referred to as flood divides, hinder the esti-11 mation of rare floods. In this paper we develop an automated methodology for the detection 12 of flood divides from observations and models, and apply it to a large set of case studies 13 in the USA and Germany. We then assess the reliability of the PHysically-based Extreme 14 Value (PHEV) distribution of river flows to identify catchments that might experience a 15 flood divide, validating its results against observations. This tool is suitable for the iden-16 tification of flood divides, with a high correct detection rate especially in the autumn and 17 summer seasons. It instead tends to indicate the emergence of flood divides not visible 18 in the observations in spring and winter. We examine possible reasons of this behavior, 19 finding them in the typical streamflow dynamics of the concerned case studies. By means 20 of a controlled experiment we also re-evaluate detection capabilities of observations and 21 PHEV after discarding the highest maxima for all cases where both empirical and theoret-22 ical estimates display flood divides. PHEV mostly confirms its capability to detect a flood 23 divide as observed in the original flood frequency curve, even if the shortened one does not 24 show it. These findings prove its reliability for the identification of flood divides and set the 25 premises for a deeper investigation of physiographic and hydroclimatic attributes controlling 26 the emergence of discontinuities in flood frequency curves. 27

# <sup>28</sup> 1 Introduction

Despite considerable efforts to achieve reliable estimation of rare floods, these events are 29 still among the most common natural disasters (Wallemacq & House, 2018). The evaluation 30 of their hazard is however crucial for several applications, including the design of hydraulic 31 structures, risk planning and mitigation, and computation of premiums in the insurance 32 industry. Appraisal of the flood hazard is especially difficult when the magnitude of the 33 rarer floods can take values which are several times to orders of magnitude larger than 34 commonly observed floods, resulting in a marked uprise of the flood frequency curve beyond 35 certain return periods (Rogger et al., 2012; Smith et al., 2018). 36

Cognitive biases often lead to downplay the occurrence of such extreme events (B. Merz et al., 2015, 2021), although the scientific literature repeatedly signalled the pervasiveness of these behaviors terming them in various ways. In fact, heavy-tailed distributions of floods (Farquharson et al., 1992; Bernardara et al., 2008; Villarini & Smith, 2010), inversions of

concavity and step changes in flood magnitude-frequency curves (Rogger et al., 2012; Guo 41 et al., 2014; Basso et al., 2016) and large values of the ratios between the maximum flood of 42 record and the sample flood with a specified recurrence time (Smith et al., 2018) and between 43 empirical high flow percentiles (Mushtaq et al., 2022) are all manifestations of a marked 44 increase of the magnitude of the rarer floods highlighted by means of different approaches. 45 To further stress the common nature of all these phenomena, in this study we favor none 46 of the previous locutions and instead label them as flood divides. The term was chosen to 47 highlight the existence of a discharge threshold which marks the rise of progressively larger 48 floods (red square in Figure 1d) and thus distinguishes between common and increasingly 49 extreme floods that may occur in river basins. 50

Rogger et al. (2012) investigated marked uprises (i.e., discontinuities in the slope) of 51 flood frequency curves, which they called step changes, by leveraging information collected 52 from field surveys in two small alpine catchments to calibrate a distributed deterministic 53 rainfall-runoff model. They suggested that step changes occur when a threshold of the 54 catchment storage capacity is exceeded, and performed a synthetic experiment (Rogger 55 et al., 2013) to examine the effect of catchment storage thresholds and combined multiple 56 controls (e.g., the temporal variability of antecedent soil storage and the size of the saturated 57 regions) on the return period of the step change. 58

Guo et al. (2014) and Basso et al. (2016) instead linked different shapes of flood fre-59 quency curves and a marked growth of the magnitude of the rarer floods to the catchment 60 water balance. The former justified these features through the aridity index (i.e., the ra-61 tio between mean annual potential evaporation and precipitation, Budyko (1974)), showing 62 that flood frequency curves characterized by increasing aridity index are steeper. The latter 63 explained them by means of the persistency index (i.e., the ratio between mean catchment 64 response time and runoff frequency, Botter et al. (2013)) and highlighted that the concavity 65 of the flood frequency curve changes from downward to upward shifting from persistent to 66 erratic regimes, thus causing the emergence of flood divides. 67

Smith et al. (2018) computed the ratio between the maximum flood of record and the sample 10-year flood for thousands of gauges across the USA, finding large values for a substantial amount of them. Different flood-generating processes (R. Merz & Blöschl, 2003; Berghuijs et al., 2014; Tarasova et al., 2020) or mixtures of flood event types (Hirschboeck, 1987; Villarini & Smith, 2010; Smith et al., 2018) were indicated by other studies as possible
causes of these marked increases of the magnitude of the rarer floods.

Finally, a rather common approach to study this phenomenon consists in evaluating 74 the shape parameter of Generalized Extreme Value distributions fitted to observed annual 75 maximum series (Farquharson et al., 1992; Bernardara et al., 2008; Villarini & Smith, 76 2010; Smith et al., 2018). Notwithstanding the drawbacks of such a parametric approach 77 applied in association with limited records of annual maxima, these studies highlighted the 78 ubiquitous occurrence of flood divides and flood distributions characterized by thick upper 79 tails, as indicated by widespread positive values of the shape parameter. Moreover, Smith et 80 al. (2018) showed that the values of the shape parameter significantly increase with longer 81 data records. Their findings thus suggest that uprises of flood frequency curves may be the 82 norm rather than rare conditions, pointing to the limited data record as the reason for the 83 latter belief. 84

Although former research hints at the ubiquitousness of flood divides in flood frequency 85 curves and provide indications of their possible drivers, a quantitative methodology to iden-86 tify flood divides, which is robust to sampling uncertainty and tested in a large set of case 87 studies, is still lacking. The relevance of our study is thus twofold: (i) we develop such a 88 methodology for the detection of flood divides and evaluate their emergence across the US 89 and Germany, in a large set of catchments with contrasting physio-climatic features; (ii) we 90 examine the reliability of a process-based stochastic framework for the estimation of flood 91 frequency curves to detect flood divides and infer their occurrence, benchmarking its results 92 against observations. 93

# <sup>94</sup> 2 Methodology and Data

# 95 2.1 The Physically-based Extreme Value distribution of river flows

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# 2.1.1 Theoretical framework

The PHysically-based Extreme Value (PHEV) distribution of river flows is a parsimonious mechanistic-stochastic formulation of flood frequency curves (Basso et al., 2016, 2021) that stems from a rigorous mathematical description of catchment-scale daily soil moisture and streamflow dynamics in river basins (Laio et al., 2001; Porporato et al., 2004; Botter et al., 2007). In this framework, daily precipitation is represented as a marked-Poisson process

with frequency  $\lambda_P [T^{-1}]$  and exponentially-distributed depths with average value  $\alpha [L]$ . Soil 102 moisture decreases due to evapotranspiration and is replenished by precipitation events that 103 eventually trigger runoff pulses when an upper wetness threshold is crossed. These pulses, 104 which feed water to a hydrologic storage, are also a Poisson process with frequency  $\lambda < \lambda_P$ 105  $[T^{-1}]$  and an exponential distribution of magnitudes with mean  $\alpha$  [L]. A non-linear (i.e., 106 power-law) storage-discharge relation epitomizes the hydrological response of the catchment 107 and encompasses the joint effect of different flow components (Brutsaert & Nieber, 1977; 108 Basso, Schirmer, & Botter, 2015). 109

The above-summarized mechanistic-stochastic description of runoff generation pro-110 cesses allows for expressing the probability distributions of daily flows (Botter et al., 2009) 111 and peak flows (i.e., local flow peaks occurring as a result of streamflow-producing rainfall 112 events) as a function of a few physically meaningful parameters (Basso et al., 2016). It 113 also enables classifying hydrologic regimes according to their typical streamflow dynamics, 114 which are summarized by the persistency index (Botter et al., 2013). This is defined as the 115 ratio between runoff frequency and mean catchment response time. An erratic regime (low 116 persistency index), which is commonly found during dry seasons, very hot humid seasons 117 with intense evapotranspiration or in fast responding catchments, is characterized by peri-118 ods between the arrival of runoff-producing rainfall events which are longer than the typical 119 duration of flow pulses. Conversely a persistent regime (high persistency index), typically 120 occurring in cold-humid seasons and lowland catchments, is characterized by frequent rain-121 fall events and a rather constant water supply to the catchment. 122

Considering that peak flows in a given reference period (e.g., a season) are Poisson distributed and postulating their independence yield the probability distribution of flow maxima (i.e., maximum values in a specified timespan). The return period is finally obtained as the inverse of the exceedance cumulative probability of flow maxima, thus providing an expression of the flood frequency curve which reads (Basso et al., 2016):

$$T_r(q) = \frac{1}{1 - \exp\left[-\lambda \tau D_j(q)\right]} \tag{1}$$

where  $\tau$  [T] is the duration in days of the reference period used in the analyses;  $D_j(q) = \int_q^{\infty} p_j(q) dq$  is the exceedance cumulative probability of peak flows;  $p_j$  is the probability density function of peak flows,  $p_j(q) = Cq^{1-a} \exp(\frac{\lambda q^{1-a}}{K(1-a)} - \frac{q^{2-a}}{\alpha K(2-a)})$ ;  $\alpha$  and  $\lambda$  are the

aforementioned parameters describing Poisson-distributed runoff events, a and K are the 131

parameters of the power-law storage-discharge relation, and C is a normalization constant. 132

#### 2.1.2 Parameter Estimation 133

The four parameters of PHEV  $(\alpha, \lambda, a, K)$  are rather straightforward to estimate 134 at the catchment scale. They are indeed directly derived from the observed time series 135 of precipitation and streamflow:  $\alpha$  is computed as the mean daily rainfall depth in rainy 136 days, while  $\lambda$  (frequency of streamflow-producing rainfall) as the ratio between the long 137 term mean daily flow  $\langle q \rangle$  and  $\alpha$  (Botter et al., 2007). The parameters of the power-law 138 storage-discharge relation (i.e., the recession exponent a and coefficient K) are estimated 139 through hydrograph recession analysis (Brutsaert & Nieber, 1977) following the approach 140 proposed by Biswal and Marani (2010). Finally, the recession coefficient is not directly used 141 as input in Eq. (1), but it is replaced by its maximum likelihood estimation on the observed 142 seasonal flood frequency curve (Basso et al., 2016). 143

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# 2.2 Identification of Flood Divides

To identify flood divides, we adopt the method proposed by Rogger et al. (2013): a flood 145 divide is defined as the sharpest bend of the flood frequency curve, here considered in terms 146 of rescaled streamflow maxima (i.e., seasonal maxima divided by the long term mean daily 147 flow,  $\langle q \rangle$ ) as a function of the return period, the latter represented in logarithmic scale. 148 We develop a methodology dedicated to its identification from both empirical estimates of 149 the flood frequency curve obtained by means of Weibull plotting position and PHEV. 150

The resulting approach, which can be employed without depending on subjective eval-151 uation, is detailed in the following. 152

1. The curvature of the flood frequency curve, of which we show an example in Figure 153 1, is computed as  $logTr''/(1 + logTr'^2)^{(3/2)}$  (where the apex indicates the derivation 154 operation with respect to the rescaled streamflow) for both the observations and 155 PHEV. In the former case, we use the method developed by Jianchun et al. (1994) 156 for computing derivatives in non-equally spaced points, while for PHEV we employ 157 the Python routine from the Scipy library (*misc.derivative*), which uses a central 158 difference formula with spacing dx to compute the  $n^{th}$  derivative at a specified point. 159

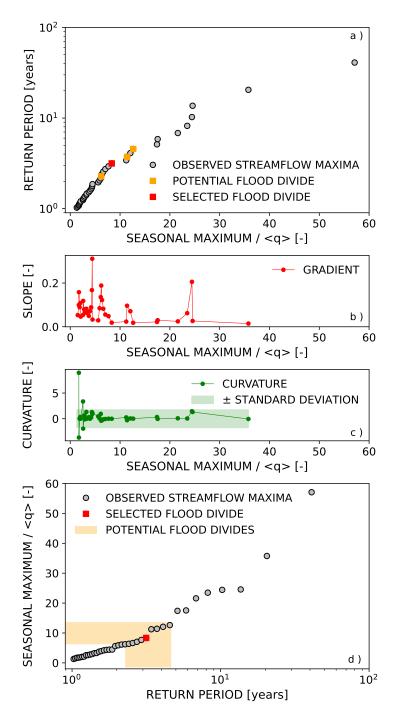


Figure 1: Exemplary application of the proposed methodology to detect flood divides to the Rott river at Kinning, Bavaria (ID: 18801005), in the summer season. a) Visualization of how the approach is actually applied, i.e., expressing the logarithm of the return period as a function of the rescaled seasonal maxima (gray filled circles). Potential flood divides (i.e., all the points with a p-value of the Mann-Whitney U-test lower than 0.05) are represented by orange squares, while the selected one (i.e., the one exhibiting the minimum p-value of the Mann-Whitney U-test and Cohen's d greater than 0.4) is depicted with a red square. b) First derivative computed on observations. c) Curvature computed on observations, with the shaded area representing twice its standard deviation. d) Standard representation of the flood frequency curve, namely observed maxima as a function of the logarithmic value of the return period (gray filled circles). The red square indicates the selected flood divide, while the orange shaded area represents the range of potential flood divides.

- 2. As the noise associated to computing the curvature on a discrete and rather sparse set of points (seasonal maxima) might lead to identification errors, a heuristic filter is applied on the curvature calculated from observations: only points on the right-hand side of the last value of the curvature exceeding the range  $\pm \sigma$  (where  $\sigma$  indicates the standard deviation of the curvature itself) are considered (Figure 1c);
- 3. The Mann-Whitney U-test (Mann & Whitney, 1947) is applied on the values of the 165 first derivatives on the left and right-hand sides of each potential flood divide identified 166 at point 2 to check if their distributions are statistically different at a significant level 167 equals to 0.05 (in other words, if the slope of the curve significantly differs between 168 the left and right-hand side of the flood divide); the effect size is then computed by 169 means of the Cohen's d (Cohen, 1974) to evaluate if the magnitude of the difference is 170 relevant (Sullivan & Feinn, 2012). For PHEV, this step is performed on a dense set of 171 values, equally spaced with an interval  $\Delta q = 0.05$  up to a value of rescaled streamflow 172 equal to 200, i.e., 200 times the long-term average streamflow. The relative increment 173 of the slope between the left and right-hand side of a potential PHEV flood divide is 174 also evaluated within the observational range. 175
- 4. We finally identify as flood divide the point for which the p-value of the Mann-Whitney test is the lowest, provided that the Cohen's *d* is greater than 0.4 (moderate effect size; Gignac and Szodorai (2016); Lovakov and Agadullina (2021)) and the slope increment exceeds a value of 1%.

Figure 1 visually exemplifies the application of the developed approach for flood divides 180 detection to the flood frequency curve of the Rott river at Kinning, Bavaria (ID: 18801005), 181 in the summer season. In Figure 1a the flood frequency curve is represented with switched 182 axes (i.e., the logarithm of the return period is represented on the y-axis whereas the rescaled 183 seasonal maxima on the x-axis), as streamflow is the independent variable in Eq. (1). The 184 red square in Figure 1a-d represents the selected flood divide, i.e., the one associated to 185 the lowest p-value of the Mann-Whitney U-test applied to the distributions of the first 186 derivatives (Figure 1b) and fulfilling the additional criterion on the Cohen's d. We also 187 show points that are initially analyzed as potential flood divides (i.e., all the points with a 188 Mann-Whitney p-value lower than 0.05, orange squares in Figure 1a). 189

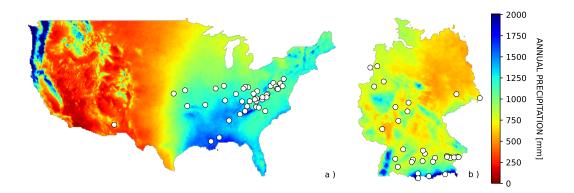


Figure 2: Select river basins (white filled circles) from the (A) MOPEX and (B) German datasets. The background of the maps represents 30-years annual precipitation normals (1981-2010 for the US and 1991-2020 for Germany).

# 190 2.3 Datasets

We use daily rainfall and streamflow time series from the Model Parameter Estimation 191 Experiment (MOPEX) dataset (Duan et al., 2005; Schaake et al., 2006) and from Germany 192 (Tarasova et al., 2018), performing all analyses in a seasonal time frame to account for 193 the seasonality of rainfall and runoff (Allamano et al., 2011; Baratti et al., 2012). To 194 assure that PHEV suitably represents the key processes of streamflow generation in the 195 set of case studies, we only consider catchments with low human impact, weak or absent 196 inter-seasonal snow dynamics (Botter et al., 2013; Wang & Hejazi, 2011) and hydrograph 197 recession properties which are independent of the peak flow (Basso et al., 2021). Similarly 198 to previous studies (R. Merz et al., 2020), we as well restrict our analysis to cases for which 199 the root mean square error (RMSE) between the predicted and observed flood frequency 200 curve is limited (i.e., lower than 0.3), as a fairly accurate estimation of the flood frequency 201 curve is a precondition to investigate if PHEV is able to correctly identify flood divides 202 and whether their occurrence is affected by physio-climatic catchment attributes. RMSE 203 is here calculated as  $\sqrt{\left[\sum_{i=1}^{N} (logTr_{ds} - logTr_{PHEV})^2\right]}/N$ , where  $Tr_{ds}$  and  $Tr_{PHEV}$  are 204 empirical and PHEV estimates of the return periods of seasonal maxima, and N is the 205 number of values in their observed sample. This selection yields a set of 101 case studies 206 (i.e., catchment-season combinations), divided into 23, 29, 23 and 26 cases respectively in 207 the spring, summer, autumn and winter seasons. Their catchment areas vary between 43 208 and  $9052 \text{ km}^2$  (median:  $865 \text{ km}^2$ ). The locations of their outlets are displayed in Figure 2. 209

# <sup>210</sup> **3 Results and Discussion**

We apply the methodology for the identification of flood divides introduced in the 211 previous section to each observed and analytic seasonal flood frequency curve, thus allowing 212 for evaluating the flood divide detection of PHEV against observations, which we consider 213 as benchmark (Figure 3). The bar plots in Figure 3 show the percentages of case studies 214 for which a flood divide is identified from both PHEV and the observational records (true 215 positives, dark green color), those which display a flood divide neither in the empirical nor 216 in the analytic flood frequency curves (true negatives, light green), the percentages of cases 217 where a flood divide is detected from the observations but not from the analytical model 218 (false negatives, red), and those where the analytical model has foreseen the occurrence of 219 a flood divide which is not confirmed by the available observations (false positives, orange). 220 The existence of both true positives and true negatives emphasizes the capability of PHEV 221 to mimic varied observed shapes of flood frequency curves (Basso et al., 2016) and to identify 222 both the presence and the absence of a flood divide. 223

The bar plots in Figure 3a and 3b differ for the criteria applied in the flood divide iden-224 tification methodology. In Figure 3a only the controls on the p-value of the Mann-Whitney 225 U-test mentioned in Section 2.2 are considered, whereas the additional requirements on the 226 effect size and slope increment are as well used in Figure 3b. True positives (dark green) pre-227 vail in the summer and autumn seasons of Figure 3a, amounting to about 60% of the cases. 228 False positives constitute instead a sizable share of the cases in spring and winter. When 229 more stringent requirements for the identification of flood divides are used, by accounting 230 for the mentioned additional criteria, the percentage of true positives decreases (Figure 3b, 231 dark green). A few cases of those shifting category become true negatives, indicating that 232 the slope of the flood frequency curve does not substantially increases on the right-hand 233 side of the potential flood divide, thus not representing a noteworthy hazard. Most of them 234 however become false positives (orange color in Figure 3b) as the identified changes of the 235 slope of the flood frequency curve are not substantial according to the limited amount of 236 available observations, whereas PHEV confirms the existence of a flood divide thanks to 237 its evaluation in an unlimited number of points. Consistent results are also found when 238 considering different significant levels for the Mann-Whitney test: the strictest the level the 239 highest the share of cases shifting between true and false positives, which once again points 240 to the unfeasibility of detecting flood divides with confidence from plain observations. 241

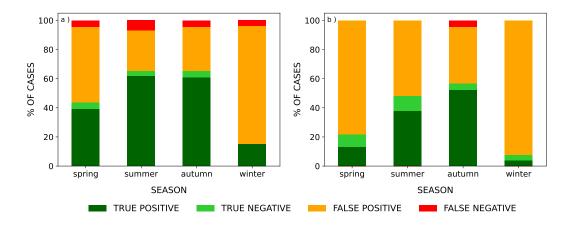


Figure 3: Performance of the PHysically-based Extreme Value (PHEV) distribution of river flows in the detection of flood divides when only the controls on the Mann-Whitney U-test are considered (see Section 2.2, panel a) and when the whole methodology for detecting flood divides is applied (see Section 2.2, panel b). Percentages are calculated on the overall number of case studies, which amount to 23, 29, 23 and 26 cases respectively in the spring, summer, autumn and winter seasons. True positives (dark green color) and true negatives (light green) indicate coherence between PHEV and observations, i.e., flood divides are either detected or not from both PHEV and the observed records. These constitute a large number of cases in summer and autumn. False positives (orange) and false negatives (red) represent the cases in which either PHEV detects a flood divide that was not identified by the observations or the observations display a flood divide which is not detected by PHEV. The reasons for the presence of false positives are further investigated in the study and clarified in the text and figures.

The predominance of false positives in spring and winter (orange color in Figure 3) 242 calls for further investigation of their causes. We therefore hypothesize that PHEV, by 243 leveraging the embedded mechanistic description of hydro-climatic dynamics taking place in 244 watersheds and the information gained from analyzing daily rainfall and streamflow series, 245 might indicate the possible emergence of flood divides that are not yet displayed by the 246 observed flood frequency curves. In fact, these empirical estimates are likely affected by 247 small sizes of the samples of large events (i.e., those on the right-hand side of each potential 248 flood divide, see Figure 1a) and by the specific character of catchments, which may have a 249 more or less enhanced propensity to exhibit extreme floods and thus display them in a limited 250 data record. We then perform the following experiment to test this hypothesis. We consider 251 the set of true positives (i.e., the 27 cases for which both PHEV as well as the observed flood 252 frequency curve show a flood divide) and retain only maxima with return periods below 5 253 years (see an explanatory example in Figure 4a, where the maxima retained are represented 254 by gray filled circles with blue contours). In so doing, we approximately discard in each case 255 the largest ten points and their corresponding years of occurrence. Thereby, fictitious flood 256 frequency curves only comprising maxima with smaller magnitudes (and return periods) 257

are created, thus reproducing the conditions we hypothesized as possible reasons of the emergence of false positives. We then apply the usual methodology for identifying flood divides on these fictitious flood frequency curves and the corresponding shortened data records.

PHEV detects a true flood divide (i.e., true positives) in 81% of the cases even when 262 the largest points are removed, whereas the observations only in 40%. The maps in Figure 263 4b and 4c summarize this result: half circles are colored either in green, if a flood divide 264 is successfully detected from the shortened flood frequency curve, or in red in the opposite 265 case. The left half of the circle depicts the detection capability of PHEV, while the right side 266 the results obtained from the observations. It can be easily seen that most left halves of the 267 circles are colored in green and most of the right ones are instead red, thus indicating a high 268 success rate of PHEV and a significantly lower one of observations in inferring the emergence 269 of flood divides from shortened records. A similar result is obtained by discarding maxima 270 with return period greater than 10 years (i.e., discarding about five-six points instead of the 271 highest ten), when PHEV correctly detects 85% of true flood divides in comparison to a 272 correct detection rate from observations of 60%. The outcome of this experiment strongly 273 suggests that the detected false positives (orange color in Figure 3) indeed arise because of 274 the statistical uncertainty of limited data records and the capability of PHEV to infer the 275 occurrence of flood divides from short series rather than by its inability to correctly identify 276 inflection points which were detected (or not) in the observed flood frequency curves. 277

A physical explanation of the reason why some observational series might not exhibit a 278 flood divide which shall be expected is provided by considering typical streamflow dynamics 279 occurring for distinct river flow regimes, here characterized by means of the persistency 280 index (Botter et al., 2013). When streamflow values weakly oscillate around their mean 281 (persistent regimes), the probability of occurrence of relatively large flows is very low, and 282 extreme events are unlikely to be captured by short time series. On the contrary, erratic 283 regimes are composed of a sequence of high flows interspersed in between prolonged periods 284 of low flows. Events which are several times (i.e., order of magnitudes) higher than the 285 average flow are thus more likely to occur in these regimes (Basso, Frascati, et al., 2015). In 286 the context of this study, false positives shall therefore mostly occur for persistent regimes, 287 as such large events enabling detection of flood divides from empirical flood frequency curves 288 are less likely to have been observed during the available data record. 289

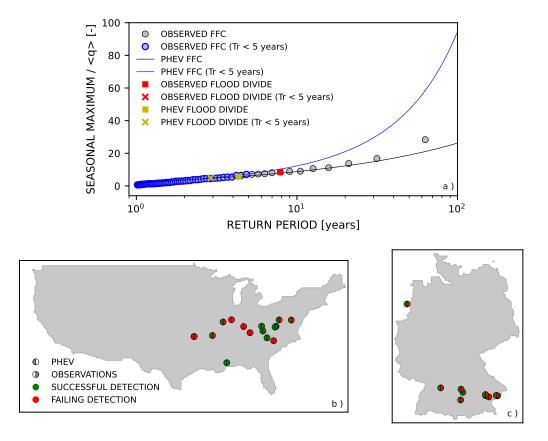


Figure 4: Visual explanation and results of an experiment aimed at testing hypotheses on the emergence of false positives. a) Gray dots with black (blue) contour represent the complete (shortened, until a return period of 5 years) observed seasonal maxima series of the Wörnitz river at Harburg, Bayern (ID:11809009), in the summer season. The solid black (blue) line displays the analytic flood frequency curve (i.e., PHEV) whose parameters are estimated from the complete (shortened) time series. The red (yellow) square indicates the flood divide detected from the observations (by PHEV) using the complete series, while the corresponding crosses (the red one is not visible in the plot as no flood divide was detected after shortening the observations) represent the observed and analytic flood divides detected on the shortened flood frequency curve. b-c) Locations of the true positives in the US (panel b) and Germany (panel c). The left (right) half of the circles represent PHEV (observations) ability to detect a flood divide when the shortened flood frequency curves (i.e., maxima characterized by return period below 5 years) are used. The green (red) colored halves indicate successful (failing) detection. Remarkably, most of the left halves are green (PHEV detects true flood divides even from the shortened series in the majority of the cases), whereas most of the right ones are red (flood divides are not always identified from observations when the shortened records are used).

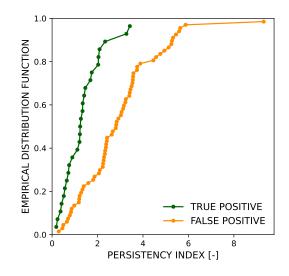


Figure 5: Empirical cumulative distribution functions of the persistency index for true positive (dark green) and false positive (orange) cases. The distributions are significantly different in a statistical sense (the p-value of the 2-samples Kolmogorov-Smirnoff test is lower than 0.01.)

In Figure 5 we compare the cumulative distributions of the persistency index for the 290 sets of true positives (dark green) and false positives (orange) to verify whether this is the 291 case. The distributions clearly differ and false positives mostly occur for persistent regimes. 292 This qualitative evaluation is validated by applying the 2-sample Kolmogorov-Smirnoff test, 293 which evaluates if two samples come from the same distribution (null-hypothesis): in this 294 case we can reject the null-hypothesis at the 0.01 significance level, meaning that the two 295 samples are drawn from different distributions and false positives are significantly more likely 296 to occur for persistent regimes. Remarkably, the seasons characterized by the larger portion 297 of false positives are spring and winter, during which regimes tend to be more persistent. 298

The physical explanation provided here of the different telling power of streamflow data 299 for rivers characterized by distinctively different streamflow dynamics agrees with the results 300 of previous research. For example, Botter et al. (2013) showed less variable streamflow 301 distributions across years in erratic regimes compared to persistent ones, which determines 302 higher representativeness of their estimates in the former case for a given length of the data 303 record. Smith et al. (2018) also demonstrated that upper tail ratios grow with the length 304 of data and, for a given data length, are larger (i.e., flood divides are more often identified) 305 in arid and semiarid regions than in humid ones. Their results jointly suggest that, given 306 similarly long data records, the typical (erratic) flow dynamics of drier areas enable more 307 reliable characterization of the whole range of values possibly spanned by streamflow and 308

of the presence or absence of flood divides according to the physical explanation providedabove.

# 311 4 Concluding Remarks

In this work we examine the occurrence of marked uprises of flood frequency curves (termed flood divides), which are pivotal for a correct estimation of river flood hazard. We develop a robust methodology to identify them from observational records and models, and evaluate the capability of the PHysically-based Extreme Value distribution of river flows (PHEV) to reliably detect flood divides.

Results show that PHEV is consistently able to recognize the presence/absence of flood 317 divides in a large set of case studies from the US and Germany. Possible reasons for the 318 occurrence of a sizeable number of false positives are investigated by accounting for both the 319 statistical uncertainty of relatively short observational records and the typical hydro-climatic 320 variability of different river basins, which affects the information content of these limited data 321 series. To this end, we perform a controlled experiment in which we remove the highest flow 322 maxima in the flood frequency curves of the true positive cases and repeat the flood divide 323 detection analysis on the shorter series, showing that PHEV can foresee the emergence of 324 flood divides even if the shortened observations do not display it. The result supports claims 325 of the dependability of flood divides initially classified as false positives. An investigation of 326 the intrinsic dynamics of streamflows in the set of true and false positives further elucidates 327 the issue. The majority of cases for which false positives are detected feature markedly 328 persistent regimes that, by their nature, rarely exhibit large extreme flow values. The 329 limited length of the available observed time series might be thus constraining the possibility 330 to observe expected flood divides, analogously to what occurs when we artificially reduce 331 the size of the observational sample. 332

The present analysis, performed on a wide set of catchments characterized by different hydroclimatic features, reveals PHEV as a reliable tool to identify and foresee the occurrence of flood divides and consequently unveil the propensity of rivers to large floods. The method is especially relevant in data scarce conditions, although limitations linked to the domain of applicability of this tools exist and have been recalled in this work. The study lays the foundations for a better comprehension of climate and landscape controls of observed marked rises of the magnitude of the rarer floods, which is the subject of ongoing research.

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